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This program was carried out under Contract DAAJ02-67-C-0093 with Whittaker Corporation, Whittaker Research and Development Division, San Diego, California.

The data contained in this report are the result of an analytical and experimental study of the stress distribution in loop and bolt joints made of composite materials. Also, an analytical study was made of the stress concentration due to a circular hole in orthotropic composite sheets.

The report has been reviewed by the U.S. Army Aviation Materiel Laboratories and is considered to be technically sound. It is published for the exchange of information and the stimulation of future research.

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APPLICATION OF MICROMECHANICS  
TO JOINTS AND CUTOUTS

Final Report

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## SUMMARY

In this report the stress distribution in loop and bolt joints made of composite materials is studied using analytical and experimental methods. Also, the stress concentration in orthotropic composite sheets with a circular hole is studied.

The numerical technique to evaluate the stresses and displacements is described. Simple design formulas to compute the stresses at critical regions are developed. A procedure of structural design is delineated and applied to specific cases.

Experimental results obtained by testing composite joints are reported. Stress distribution tests were performed using the photoelastic technique. Failure location and ultimate loads for several composite joints are shown.

This report contains the analysis of the stress concentration that appears around a circular hole in an orthotropic composite sheet under simple tension and simple shear loadings.

## FOREWORD

This report was prepared by Whittaker Research & Development/San Diego\*, San Diego, California, under Contract DAAJ02-67-C-0093, Task 1F162204A17002, for the U.S. Army Aviation Materiel Laboratories, Fort Eustis, Virginia. Work was accomplished under the direction of Mr. A. J. Gustafson, Army Project Officer.

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# LIST OF SYMBOLS

$A$	total area
$A_a, A_b, A_c$	transverse areas of the layers $a, b, c$
$A, B$	constants
$[A]$	geometric matrix (6x6)
$[A^*]$	diagonal submatrix of $[A]$ , (3x3)
$[A_{ij}]$	submatrices of $[A]$
$a$	radius of the hole at a bolt joint
$a_1, a_2, \dots, a_6$	constants
$\{a\}$	vector of the constants $a_1, a_2, \dots, a_6$ (6x1)
$b_{ij}$	inverse elastic constants
$[C]$	matrix of the elements $c_{ij}$ (3x3)
$[C_a], [C_b], [C_c]$	matrices of the elastic constants at $a, b, c$ , referred to the axis $x, y$
$[C']$	matrix of the elastic constants referred to the axis $x_a, y_a$ ( $x_b, y_b$ ; $x_c, y_c$ , ...)
$c_{ij}$	elastic constants of the material
$[D]$	location matrix (3x6)
$e$	base of the natural logarithm
$f$	function of the complex variable $(Z+Y_1 \bar{Z})$
$g$	function of the complex variable $(Z+Y_2 \bar{Z})$
$[I]$	unit matrix
$i$	imaginary unit, $i = \sqrt{-1}$
$[K]$	stiffness matrix of an element (6x6)
$[K^*]$	total stiffness matrix
$[K_{ij}^*]$	submatrices of $[K^*]$
$k_{ij}^*$	element of the total stiffness matrix
$l, m, n$	cosine directors of the directions $a, b, c$
$P, Q$	boundary load functions
$P^*$	failure load
$P_1 \xi, P_2 \xi, \dots, P_3 \eta$	nodal forces

# LIST OF SYMBOLS (Continued)

$\{P\}$	vector of the nodal forces (6x1)
$\{P^*\}$	vector of the nodal forces for the whole structure
$\{P_1^*\}$	vector of the unknown nodal forces
$\{P_2^*\}$	vector of the known nodal forces
$\{P'\}$	vector of the rotated nodal forces
$\{P_a\}, \{P_b\}, \{P_c\}$	forces at the layers a, b, c
$R_1, R_2$	moduli of $X_1, X_2$
$r, \theta$	polar coordinates
$T_x$	uniform tension at the x-direction
$T_{xy}$	shear loading
$[T]$	transformation matrix
$[T_1]$	transformation matrix of stress vector (3x3)
$[T_2]$	transformation matrix of strain vector (3x3)
$t$	thickness
$u_r$	displacement at the radial direction
$u_x, u_y$	displacements at the x- and y-directions, respectively
$V_F$	volumetric percentage of fibers in the composite
$W$	strain energy
$X_1, X_2$	auxiliary complex variables
$z$	complex variable
$\bar{z}$	conjugate of $z$
$\alpha_i$	orientation angles of the nodal directions
$\alpha_1, \alpha_2$	constants dependent upon $b_{ij}$
$[\alpha]$	inverse matrix of $[A]$
$[\alpha_{ij}]$	submatrices of $[\alpha]$
$\gamma_1, \gamma_2$	constants dependent upon $\alpha_1, \alpha_2$
$\Delta$	determinant of $[A^*]$
$\{\delta\}$	displacement vector (6x1) of the nodal displacements
$\{\delta^*\}$	vector of the nodal displacements for the whole structure
$\{\delta_1^*\}$	vector of the known nodal displacements

# LIST OF SYMBOLS (Continued)

$\{\delta_2^*\}$	vector of the unknown nodal displacements
$\{\delta'\}$	vector of the rotated nodal displacements
$\epsilon_x, \epsilon_y, \gamma_{xy}$	strain components
$\epsilon'_x, \epsilon'_y, \gamma'_{xy}$	stresses referred to the axis $x', y'$
$\{\epsilon\}$	strain vector (3x1)
$u$	factor of safety
$\xi, \eta$	local coordinates of the element
$\sigma_n$	normal stress component at the boundary
$\sigma_x, \sigma_y, \tau_{xy}$	stress components
$\sigma_r^*, \sigma_\theta^*$	individual failure stresses at $r$ - and $\theta$ -directions
$\sigma_x^*, \sigma_y^*, \tau_{xy}^*$	individual failure stresses
$\sigma'_x, \sigma'_y, \tau'_{xy}$	stresses referred to the axis $x', y'$
$\sigma$	stress vector (3x1)
$\{\sigma_a\}, \{\sigma_b\}, \{\sigma_c\}$	stress vectors at the layers $a, b, c$
$\tau_{nt}$	tangential stress component at the boundary
$\tau_{r\theta}$	shear stress referred to the directions $r, \theta$
$\Phi$	Airy function
$\Phi_1, \Phi_2$	arguments of $X_1, X_2$
$\psi$	auxiliary function
$\varphi_a, \varphi_b, \varphi_c$	orientation angles for the layers $a, b, c$
$\Omega$	area of the element

The symbols (') and (") indicate first and second total differentiation, respectively.

The symbols  $\text{Re}[ ]$  and  $\text{Im}[ ]$  indicate the real and imaginary parts of the quantity within the brackets, respectively.

## INTRODUCTION

In this investigation, methods were developed for calculating and designing joints of composite materials that have directional properties. The introduction of loads into structures, especially into thin sheets, historically has presented problems, even in isotropic materials. At the beginning of this century, such difficulties were overcome for the case of isotropic materials. Until now, however, some simplifying assumptions for joints of isotropic materials have been, in general, tacitly accepted.

In this work, for example, we found that the assumption of contact stresses in the form of a cosine function in bolt joints yields unrealistic results, even in isotropic materials. For the present investigation, the contact stresses were not assumed but rather were an outcome contained in the method of solution.

First, a stiffness matrix was developed by which composites having not only one but also  $n$ -directions of fiber layers could be taken into account. On the basis of this development, a computer program was established through which the stresses and displacements in a joint of arbitrary shape can be found. To illustrate, three sets of bolt joints fabricated from glass fiber or boron fiber composites and an isotropic material (e.g., steel) were calculated and studied in detail for several bolt diameter-to-loop thickness relations. Eighteen bolt joints were manufactured, tested, and compared with the computer results. Finally, very simple relations for the maximum stresses were established.

A similar approach was taken to the calculation of stresses in cutouts in composite materials. It was found that, in the contours of a circular hole cut into a unidirectional fiber composite, the stress concentrations are about twice that encountered in isotropic materials for certain types of loading. For this reason, a hole in a fibrous composite must be designed so that the fibers do not terminate in the hole but rather lead around the hole in an uninterrupted, organic fashion. If this is done, then cutouts in fibrous composite materials will be highly efficient compared to their counterparts in isotropic materials.

Stress distributions in the fibers and in the resin matrix can be found by using the stresses determined in this work as boundary conditions for micromechanical analysis. Such analyses were performed under earlier Army contracts concerned with oblique loading of a fiber reinforced composite.<sup>1,2,3</sup>



## DISCUSSION

### METHOD FOR THE STRESS ANALYSIS OF ANISOTROPIC JOINTS OF ARBITRARY FORM

The problem is to find the stress distribution at a joint composed of an anisotropic material and of variable thickness (Figure 1).

In general, the boundary conditions are mixed; in other words, displacements or stresses are given along the boundaries.

To find the solution, we divide the joint in a certain finite number of triangles and assume that the displacements are linear functions at each one of these small triangles. Compatibility and equilibrium equations for each node of this irregular net lead us to a system of algebraic equations that permit us to obtain the displacements at each node and, with this, to determine the stress.

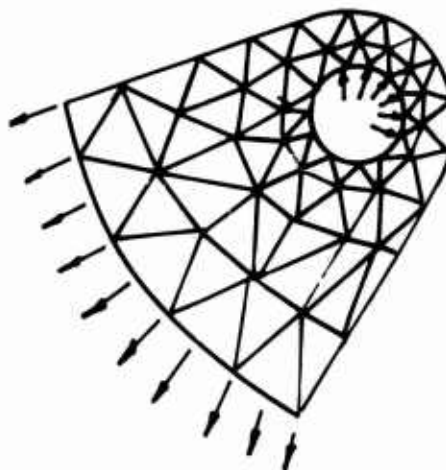


Figure 1. Subdivision of a Joint Into Finite Elements.

### Stiffness Matrix of a Triangular Element

For a triangular element (Figure 2), we assume the displacements function.<sup>4,5,6</sup>

$$\begin{aligned}u_x &= a_1 + a_2\xi + a_3\eta \\u_y &= a_4 + a_5\xi + a_6\eta\end{aligned}\tag{1}$$

where  $a_i$  are constants.

The displacements of the nodes 1, 2, and 3 are given by

$$\begin{aligned}u_{x1} &= a_1 \\u_{x2} &= a_1 + a_2\xi_2 + a_3\eta_2\end{aligned}$$

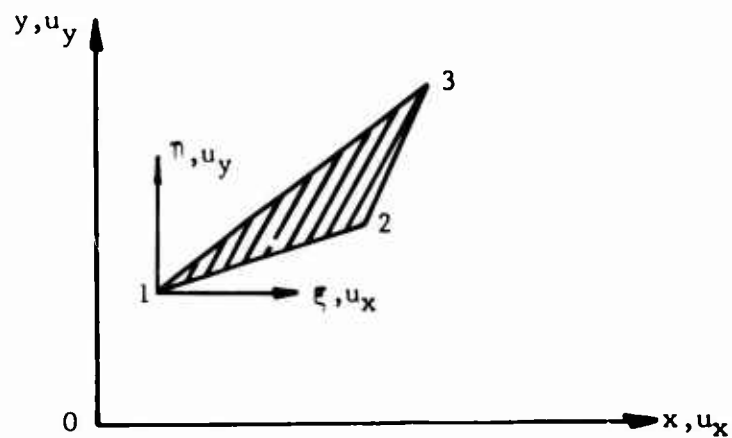


Figure 2. Triangular Element.

$$\begin{aligned}
 u_{x3} &= a_1 + a_2 \xi_3 + a_3 \eta_3 \\
 u_{y1} &= a_4 \\
 u_{y2} &= a_4 + a_5 \xi_2 + a_6 \eta_2 \\
 u_{y3} &= a_4 + a_5 \xi_3 + a_6 \eta_3
 \end{aligned} \tag{2}$$

where  $\xi, \eta$  is the local coordinate system, with origin at point 1. In matrix form, the equations (2) are

$$\begin{Bmatrix} u_{x1} \\ u_{x2} \\ u_{x3} \\ u_{y1} \\ u_{y2} \\ u_{y3} \end{Bmatrix} = \begin{bmatrix} 1 & 0 & 0 & 0 & 0 & 0 \\ 1 & \xi_2 & \eta_2 & 0 & 0 & 0 \\ 1 & \xi_3 & \eta_3 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & 0 & 0 \\ 0 & 0 & 0 & 1 & \xi_2 & \eta_2 \\ 0 & 0 & 0 & 1 & \xi_3 & \eta_3 \end{bmatrix} \begin{Bmatrix} a_1 \\ a_2 \\ a_3 \\ a_4 \\ a_5 \\ a_6 \end{Bmatrix} \tag{3}$$

or

$$\{\delta\} = [A] \{a\} \quad (4)$$

From this equation,

$$\{a\} = [A]^{-1} \{\delta\} \quad (5)$$

To find the inverse of the matrix  $[A]$ , we write

$$[A] [\alpha] = [I] \quad , \quad [\alpha] = [A]^{-1}$$

and partition in the following manner:

$$\begin{bmatrix} [A_{11}] & [A_{12}] \\ [A_{21}] & [A_{22}] \end{bmatrix} \cdot \begin{bmatrix} [\alpha_{11}] & [\alpha_{12}] \\ [\alpha_{21}] & [\alpha_{22}] \end{bmatrix} = \begin{bmatrix} [I] & [0] \\ [0] & [I] \end{bmatrix} \quad (6)$$

Taking

$$\begin{aligned} [A_{11}] &= [A_{22}] = [A^*] = \begin{bmatrix} 1 & 0 & 0 \\ 1 & \xi_2 & \eta_2 \\ 1 & \xi_3 & \eta_3 \end{bmatrix} \\ [A_{21}] &= [A_{12}] = [0] \end{aligned} \quad (7)$$

equation (6) gives

$$\begin{aligned} [A^*] \cdot [\alpha_{11}] &= [A^*] \cdot [\alpha_{22}] = [I] \\ [A^*] \cdot [\alpha_{12}] &= [0] \\ [A^*] \cdot [\alpha_{21}] &= [0] \end{aligned}$$

Thus, we obtain the following:

$$\begin{aligned} [\alpha_{11}] &= [\alpha_{22}] = [A^*]^{-1} \\ [\alpha_{21}] &= [\alpha_{12}] = [0] \end{aligned} \quad (8)$$

Then, inverting the matrix  $[A^*]$  given in equation (7), we obtain

$$[A^*]^{-1} = \begin{bmatrix} \frac{\xi_2 \eta_3 - \xi_3 \eta_2}{\Delta} & 0 & 0 \\ -\frac{\eta_{32}}{\Delta} & \frac{\eta_3}{\Delta} & -\frac{\eta_2}{\Delta} \\ \frac{\xi_{32}}{\Delta} & -\frac{\xi_3}{\Delta} & \frac{\xi_2}{\Delta} \end{bmatrix} \quad (9)$$

with  $\Delta = \xi_2 \eta_3 - \xi_3 \eta_2$  (10)

$$\xi_{ij} = \xi_i - \xi_j \quad ; \quad \eta_{ij} = \eta_i - \eta_j \quad (i, j = 1, 2, 3) \quad (11)$$

Thus, the inverse of the matrix  $[A]$  is

$$[A]^{-1} = \begin{bmatrix} [A^*]^{-1} & [0] \\ [0] & [A^*]^{-1} \end{bmatrix} \quad (12)$$

where  $[A^*]^{-1}$  is given by equation (9).

From equations (1), it is possible to obtain the strains

$$\begin{aligned} \epsilon_x &= \frac{\partial u_x}{\partial \xi} = a_2 \\ \epsilon_y &= \frac{\partial u_y}{\partial \eta} = a_6 \\ \gamma_{xy} &= \frac{\partial u_x}{\partial \eta} + \frac{\partial u_y}{\partial \xi} = a_3 + a_5 \end{aligned} \quad (13)$$

or, in matrix form,

$$\begin{Bmatrix} \epsilon_x \\ \epsilon_y \\ \gamma_{xy} \end{Bmatrix} = \begin{bmatrix} 0 & 1 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 1 \\ 0 & 0 & 1 & 0 & 1 & 0 \end{bmatrix} \cdot \begin{Bmatrix} a_1 \\ a_2 \\ a_3 \\ a_4 \\ a_5 \\ a_6 \end{Bmatrix} \quad (14)$$

Symbolically, we write equation (14) in the form

$$\{\epsilon\} = [D] \cdot \{a\} \quad (15)$$

Substituting  $\{a\}$  given by equation (5) into the last equation, we obtain

$$\{\epsilon\} = [D] \cdot [A]^{-1} \{\delta\} \quad (16)$$

On the other hand, the relationship between stresses and strains for an anisotropic material and the two-dimensional problem is

$$\begin{Bmatrix} \sigma_x \\ \sigma_y \\ \tau_{xy} \end{Bmatrix} = \begin{bmatrix} c_{11} & c_{12} & c_{13} \\ c_{12} & c_{22} & c_{23} \\ c_{13} & c_{23} & c_{33} \end{bmatrix} \begin{Bmatrix} \epsilon_x \\ \epsilon_y \\ \gamma_{xy} \end{Bmatrix} \quad (17)$$

or

$$\{\sigma\} = [C] \{\epsilon\} \quad (18)$$

Putting equation (16) into equation (18), we get

$$\{\sigma\} = [C] [D] [A]^{-1} \{\delta\} \quad (19)$$

The strain energy is given by

$$\begin{aligned}
 W &= \frac{1}{2} \int_V (\epsilon_x \sigma_x + \epsilon_y \sigma_y + \gamma_{xy} \tau_{xy}) dv \\
 &= \frac{1}{2} \iint t [\epsilon_x \quad \epsilon_y \quad \gamma_{xy}] \cdot \begin{Bmatrix} \sigma_x \\ \sigma_y \\ \tau_{xy} \end{Bmatrix} dx dy = \frac{1}{2} \iint t \{\epsilon\}^T \cdot \{\sigma\} dx dy
 \end{aligned} \tag{20}$$

where  $t$  is the thickness of the joint and  $\{\epsilon\}^T$  is the transposed vector of  $\{\epsilon\}$ . But

$$\begin{aligned}
 \{\epsilon\}^T &= ([D] \cdot \{a\})^T = \{a\}^T [D]^T \\
 &= ([A]^{-1} \cdot \{\delta\})^T [D]^T = \{\delta\}^T ([A]^{-1})^T [D]^T
 \end{aligned} \tag{21}$$

where we used equations (15) and (5). With equations (19) and (21), the strain energy is expressed by

$$W = \frac{1}{2} \iint t \{\delta\}^T ([A]^{-1})^T [D]^T [C] [D] [A]^{-1} \{\delta\} dx dy \tag{22}$$

Now we can assume that the forces  $P_{1\xi}$ ,  $P_{2\xi}$ ,  $P_{3\xi}$ ,  $P_{1\eta}$ ,  $P_{2\eta}$ , and  $P_{3\eta}$  are applied on the triangle vertex, as shown in Figure 3.

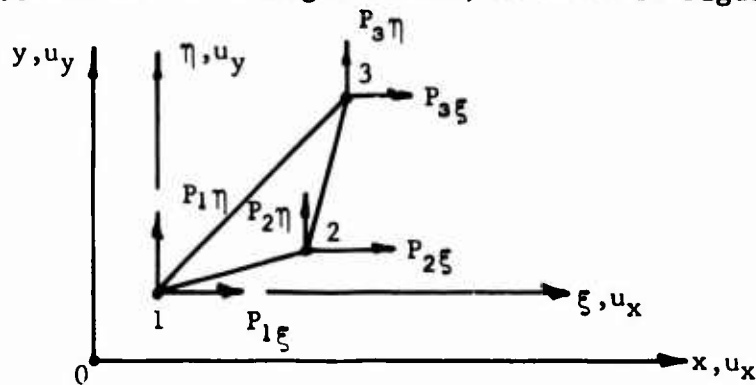


Figure 3. Generalized Nodal Forces.

We can define a stiffness coefficient  $k_{ij}^*$  as the force that is necessary to apply at  $i$  direction when the displacement at  $j$  direction is the unit and all other displacements are zero. The directions  $i$  and  $j$  can be any one of  $P_1\xi$ ,  $P_2\xi$ , ...,  $P_3\eta$ . If, for example,  $u_{x_2} = 1$  and  $u_{x_1} = u_{x_3} = u_{y_1} = u_{y_2} = u_{y_3} = 0$ , the stiffness coefficients  $k_{i2}$  are the forces that we need to apply at directions of  $P_1\xi$ ,  $P_2\xi$ , ...,  $P_3\eta$ , to keep this deformation of the triangle.

To compute the stiffness coefficients, we will use the second Castigliano theorem, which permits us to find the generalized force by differentiation of the total strain energy with respect to the corresponding displacement. Thus,

$$P_i = \frac{\partial W}{\partial \delta_i} \quad (23)$$

Then, by differentiation of equation (22) with respect to  $\{\delta\}$ , we have

$$\{P\} = \int \int t \left( [A]^{-1} \right)^T [D]^T [C] [D] [A]^{-1} dx dy \cdot \{\delta\} \quad (24)$$

and the stiffness matrix  $[K]$  yields

$$[K] = \int \int t \left( [A]^{-1} \right)^T [D]^T [C] [D] [A]^{-1} dx dy \quad (25)$$

Assuming that the thickness of the triangle is constant, the stiffness matrix becomes

$$[K] = \left( [A]^{-1} \right)^T [D]^T [C] [D] [A]^{-1} \cdot t \Omega \quad (26)$$

with  $\Omega$  equal to the area of the triangle.

Equation (24) can be written as

$$\{P\} = [K] \{\delta\} \quad (27)$$

which is referred to the axis  $x, y$  or, in other words,  $\xi, \eta$ . However, for certain special mixed boundary conditions, it would be useful to

refer equation (27) to the rotated axis. The rotations are  $\alpha_1$ ,  $\alpha_2$ , and  $\alpha_3$  at the nodes 1, 2, and 3, respectively, as shown in Figure 4.

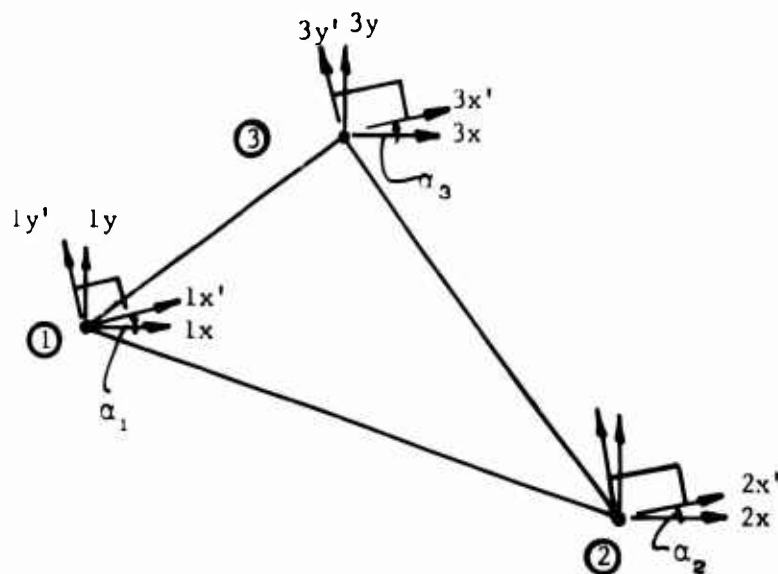


Figure 4. Rotation of the Nodal Directions.

The rotated forces vector  $\{P'\}$  is related to  $\{P\}$  as

$$\begin{Bmatrix} P_{1x} \\ P_{2x} \\ P_{3x} \\ P_{1y} \\ P_{2y} \\ P_{3y} \end{Bmatrix} = \begin{bmatrix} \cos \alpha_1 & 0 & 0 & -\sin \alpha_1 & 0 & 0 \\ 0 & \cos \alpha_2 & 0 & 0 & -\sin \alpha_2 & 0 \\ 0 & 0 & \cos \alpha_3 & 0 & 0 & -\sin \alpha_3 \\ \sin \alpha_1 & 0 & 0 & \cos \alpha_1 & 0 & 0 \\ 0 & \sin \alpha_2 & 0 & 0 & \cos \alpha_2 & 0 \\ 0 & 0 & \sin \alpha_3 & 0 & 0 & \cos \alpha_3 \end{bmatrix} \begin{Bmatrix} P'_{1x} \\ P'_{2x} \\ P'_{3x} \\ P'_{1y} \\ P'_{2y} \\ P'_{3y} \end{Bmatrix} \quad (28)$$

or, in condensed form,

$$\{P\} = [T] \{P'\} \quad (29)$$



Analogously, we can write

$$\{\dot{\epsilon}\} = [T] \{\delta'\} \quad (30)$$

By substituting equations (29) and (30) into equation (27), we obtain

$$\{P'\} = [K'] \{\delta'\} \quad (31)$$

where  $[K']$  is the rotated stiffness matrix given by

$$[K'] = [T]^{-1} [K] [T] \quad (32)$$

The total stiffness matrix of the whole structure can be obtained by summing in adequate order the elements of the matrices  $[K']$  of each element. The law of formation for the elements of the total stiffness matrix  $[K^*]$  is

$$k_{ij}^* = \sum_{n=1}^N k'_{ij}{}^n \quad (33)$$

where  $k_{ij}^*$  is an element of  $[K^*]$  and  $k'_{ij}{}^n$  is the element of the matrix  $[K']$  for the triangle  $n$  after the conversion of indices  $i, j$  from the indices 1 through 6 to the actual indices of the whole structure. The summation is extended to the  $N$  triangles.

If the numbering of the nodal directions is started by the directions in which the displacement is known, the matrix equilibrium equation for the whole structure

$$[K^*] \{\delta^*\} = \{P^*\} \quad (34)$$

can be partitioned as

$$\begin{bmatrix} [K_{11}^*] & [K_{12}^*] \\ [K_{21}^*] & [K_{22}^*] \end{bmatrix} \begin{Bmatrix} \{\delta_1^*\} \\ \{\delta_2^*\} \end{Bmatrix} = \begin{Bmatrix} \{P_1^*\} \\ \{P_2^*\} \end{Bmatrix} \quad (35)$$

where  $\{\delta_1^*\}$  is the vector of the known nodal displacements  
 $\{\delta_2^*\}$  is the vector of the unknown nodal displacements  
 $\{P_1^*\}$  is the vector of the unknown nodal forces  
 $\{P_2^*\}$  is the vector of the known nodal forces

The second row of equation (35) can be written as

$$[K_{22}^*] \{\delta_2^*\} = \{P_2^*\} - [K_{21}^*] \{\delta_1^*\} \quad (36)$$

This is a system of linear equations that, when solved, enables us to know the displacements  $\{\delta_2^*\}$ . Once that  $\{\delta_2^*\}$  is known, the first row of equation (35) gives the nodal forces  $\{P_1^*\}$ .

When all the nodal displacements are known, the stresses can be determined by substituting the displacements  $\{\delta\}$ , corresponding to each triangle, in equation (19).

It must be noted that the stresses become constant at each triangle. Thus, to have a good approximation, the triangular net must be more dense in those places where the stress gradient is greater, as occurs in areas of stress concentrations.

The stress diagrams can be smoothed out adequately by taking the average of stresses between two adjacent triangles and assigning this average to the midpoint of the common side.

#### Computation of the Matrix $[C]$ Elements

In the most general case we can assume that the material contains, at an arbitrary point, fibers in  $n$ -directions. For the following development, we take only three different directions,  $a$ ,  $b$ , and  $c$ , but the generalization to any number of directions is immediate.

We refer the material to the axes  $x$  and  $y$  (Figure 5). If we impose a deformation  $\{\epsilon\}$ , the materials with fibers at directions  $a$ ,  $b$ , and  $c$  each have the stresses

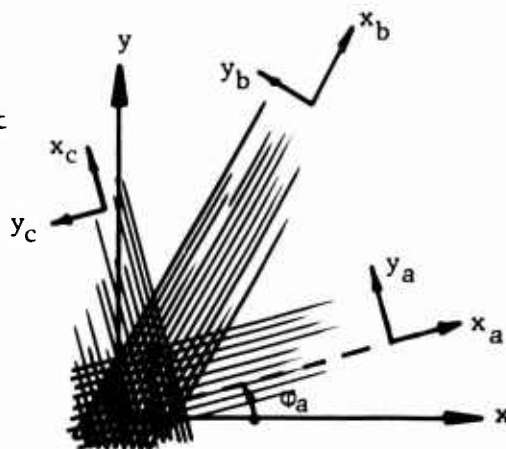


Figure 5. Multilayered Composite.

$$\{\sigma_a\} = [C_a]\{\epsilon\} \quad , \quad \{\sigma_b\} = [C_b]\{\epsilon\} \quad , \quad \{\sigma_c\} = [C_c]\{\epsilon\} \quad (37)$$

where the vectors  $\{\sigma_a\}$ ,  $\{\sigma_b\}$ , and  $\{\sigma_c\}$  and the matrices  $[C_a]$ ,  $[C_b]$ , and  $[C_c]$  are referring to axes  $x$  and  $y$ .

Considering a certain total area  $A$  composed of the areas  $A_a$ ,  $A_b$ , and  $A_c$  corresponding to the three materials, that is to say,

$$A = A_a + A_b + A_c \quad (38)$$

the forces produced by the imposed deformation are

$$\begin{aligned} \{P_a\} &= A_a \{\sigma_a\} = A_a [C_a] \{\epsilon\} \\ \{P_b\} &= A_b \{\sigma_b\} = A_b [C_b] \{\epsilon\} \\ \{P_c\} &= A_c \{\sigma_c\} = A_c [C_c] \{\epsilon\} \end{aligned} \quad (39)$$

Then the total force is

$$\{P\} = \{P_a\} + \{P_b\} + \{P_c\}$$

or, by using equations (39),

$$\{P\} = \left[ A_a [C_a] + A_b [C_b] + A_c [C_c] \right] \{\epsilon\} \quad (40)$$

On the other hand, it is possible to assume that  $\{P\}$  is produced by a stress  $\{\sigma\}$  applied on the total area  $A$ ,

$$\{P\} = A \{\sigma\} \quad (41)$$

Comparing equations (40) and (41), we find

$$\{\sigma\} = \left( \frac{A_a}{A} [C_a] + \frac{A_b}{A} [C_b] + \frac{A_c}{A} [C_c] \right) \{\epsilon\} \quad (42)$$

Thus we arrive at a matrix  $[C]$  given by

$$[C] = n_a [C_a] + n_b [C_b] + n_c [C_c] \quad (43)$$

$$\text{where } n_a = \frac{A_a}{A}, \quad n_b = \frac{A_b}{A}, \quad n_a + n_b + n_c = 1 \quad (44)$$

If the matrix material (resin) is the same for the three materials, which is the most common case, the numbers  $n_a$ ,  $n_b$ , and  $n_c$  are proportional to the number of fibers (or layers) at each direction  $a$ ,  $b$ , or  $c$ .

Referring the elastic constants for the material  $a$  to the axis  $x_a$ ,  $y_a$  (Figure 5), we have<sup>7</sup>

$$\begin{Bmatrix} \sigma'_x \\ \sigma'_y \\ \tau'_{xy} \end{Bmatrix} = \begin{bmatrix} c'_{11} & c'_{12} & 0 \\ c'_{12} & c'_{22} & 0 \\ 0 & 0 & c'_{33} \end{bmatrix} \begin{Bmatrix} \epsilon'_x \\ \epsilon'_y \\ \gamma'_{xy} \end{Bmatrix} \quad (45)$$

because  $x_a$ ,  $y_a$  are principal axes of orthotropy for the material  $a$ . Equation (45) can be written in briefer form, as follows:

$$\{\sigma'_a\} = [C'_a] \{\epsilon'_a\} \quad (46)$$

Now we want to transfer equation (46) from axis  $x_a, y_a$  to axis  $x, y$ . The stresses are transformed by the following equation:

$$\{\sigma'_a\} = [T_{1a}] \{\sigma_a\} \quad (47)$$

where

$$[T_{1a}] = \begin{bmatrix} l_a^2 & m_a^2 & 2l_a m_a \\ m_a^2 & l_a^2 & -2l_a m_a \\ -l_a m_a & l_a m_a & l_a^2 - m_a^2 \end{bmatrix} \quad (48)$$

and

$$l_a = \cos \varphi_a, \quad m_a = \sin \varphi_a \quad (49)$$

The strains are transformed by

$$\{\epsilon'_a\} = [T_{2a}] \{\epsilon_a\} \quad (50)$$

with

$$[T_{2a}] = \begin{bmatrix} l_a^2 & m_a^2 & l_a m_a \\ m_a^2 & l_a^2 & -l_a m_a \\ -2l_a m_a & 2l_a m_a & l_a^2 - m_a^2 \end{bmatrix} \quad (51)$$

Substituting equations (47) and (50) into equation (46) and then multiplying both members by  $[T_{1a}]^{-1}$  gives us

$$\{\sigma_a\} = [T_{1a}]^{-1} [C'_a] [T_2] \{\epsilon\} \quad (52)$$

Thus, by comparison of equations (52) and the first equation (37), we find

$$[C_a] = [T_{1a}]^{-1} [C'_a] [T_{2a}] \quad (53)$$

In the same way, it is possible to find matrices  $[C_b]$  and  $[C_c]$ , using the transformation matrix  $[T_{1b}]$ ,  $[T_{1c}]$ ,  $[T_{2b}]$ , and  $[T_{2c}]$ .

Thus, the following steps are necessary to find matrix  $[C]$  of a triangle:

1. Establish angle orientations  $\varphi_a$ ,  $\varphi_b$ , and  $\varphi_c$  of the fibers.
2. Establish the fiber proportions at each direction  $n_a$ ,  $n_b$ , and  $n_c$ .
3. On the base of the constants for each layer, obtain  $[C_a]$ ,  $[C_b]$ , and  $[C_c]$  by using equation (53).
4. By applying equation (43), compute  $[C]$ .

The elastic constants for each layer can be obtained with the formulas for unidirectional composites based on the fiber volumetric content and the elastic properties of the fiber and matrix material.

#### Computer Program

With the elements developed in this section, a computer program was devised which enabled us to establish the stresses and displacements for a joint. The shape of the joint can be arbitrary as well as the system of applied loads. In other words, the joint can be solved under arbitrary mixed boundary conditions. This program also covered a very

specific boundary condition that appears when the displacement component at the normal direction of a curved boundary is restricted and also when nonfriction is prescribed.

The thickness of the joint can be variable. The analysis can be performed for any anisotropic material with variable elastic properties.

### STRESS ANALYSIS OF BOLT JOINTS

#### Basic Assumptions

The stress analysis performed during this contract was concerned primarily with finding the stresses in a bolt joint under tension (Figure 6).

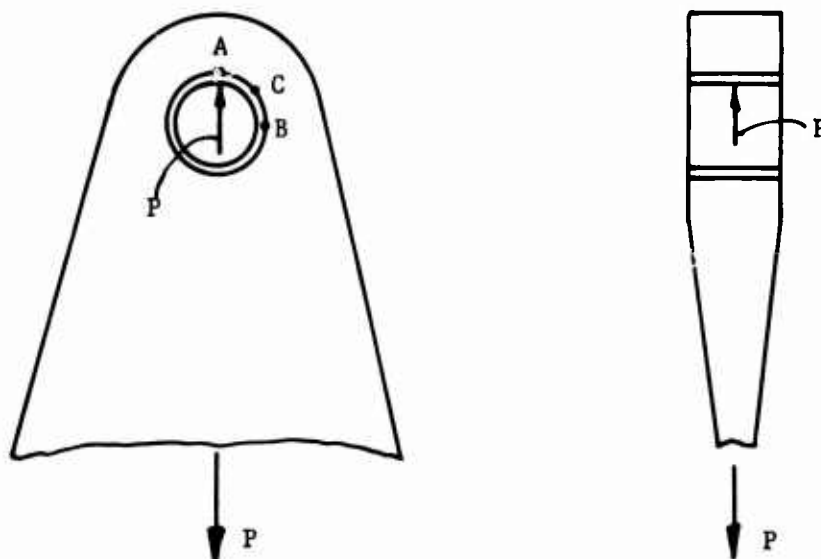


Figure 6. Typical Joint.

Between the bolt and the joint there is a bushing of bronze or steel which prevents the composite material from coming into direct contact with the steel bolt.

Critical points in a stress analysis of this type of joint are the boundary conditions to be taken along the hole contour in contact with the bushing. Many investigators adopted the cosine law for the pressure distribution on the bolt.<sup>8</sup> However, the stress distribution obtained on the

basis of this assumption does not correlate with the experimental results, especially in the prediction of the point of initial failure. In fact, if the hypothesis of the cosine law pressure distribution is adopted, the critical point is A in Figure 6 while the actual point is indicated at a position such as C. A more realistic stress distribution is obtained by assuming that the radial displacement on the boundary ACB is zero, and that there is no friction between bolt and joint. The pressure distribution on the hole boundary obtained from this hypothesis shows a considerable difference with respect to the cosine distribution. Thus, the boundary conditions adopted in the following analysis are those depicted in Figure 7.

1. On the external boundary:

$$\sigma_n = \tau_{nt} = 0$$

where n,t indicates the normal and tangential directions, respectively.

2. On the hole boundary ACB:  $\sigma_r = \tau_{r\theta} = 0$

$$u_r = 0$$

$$\tau_{r\theta} = 0$$

with r and  $\theta$ , polar coordinates, as indicated in Figure 7.

3. On the hole boundary ADB:

$$\sigma_r = \tau_{r\theta} = 0$$

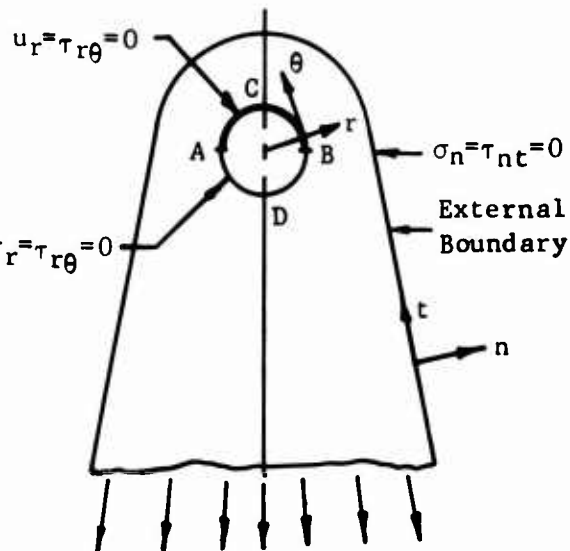


Figure 7. Boundary Conditions.

These boundary conditions correspond to a tension loading on the joint.

### Stress Distributions

The stress distribution of joints was established by using the computer program developed on the basis of the elements given in the preceding section. A parametric study was conducted to find the differences between the stress distribution obtained in an isotropic joint and those in fiber reinforced joints, both with the same geometric characteristics.

A joint was selected with a ratio  $t/a = 1.02$  (see Figure 8), which is used frequently in the applications. The following figures show the radial and tangential stresses  $\sigma_r$  and  $\sigma_\theta$  in the joint for different materials. The stresses  $\sigma_r$  are all compression and  $\sigma_\theta$  are all tension. The shear stress  $\tau_{r\theta}$  can be considered zero for practical purposes. Thus, the stress trajectories of tension are approximately concentric circles, and the compression trajectories are radial straight lines. The stress diagrams shown in the figures correspond to the following cases:

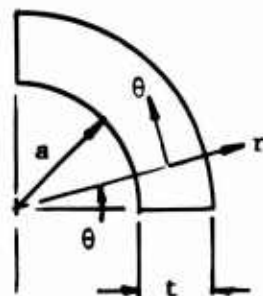


Figure 8. Dimensions and Coordinate Axis.

Figure No.	Material
9	Isotropic (steel)
10	Unidirectional glass composite with $V_F = 0.6$
11	Unidirectional glass composite with $V_F = 0.7$
12	Unidirectional glass composite with $V_F = 0.8$
13	Unidirectional boron composite with $V_F = 0.7$

The term unidirectional composite indicates that all the fibers in the part being studied (Figure 8) are parallel to the circular boundaries.  $V_F$  indicates the volume content of fibers.

The stresses plotted in Figures 9 through 13 correspond to a total applied tension force  $P = 2t$ ; in other words, the average stress  $(\sigma_\theta)_{\theta=0}$  is unity. From the analysis of these stress diagrams, it is possible to draw the following conclusions:

1. The peaks of both stresses  $\sigma_r$  and  $\sigma_\theta$  are on the internal boundary.
2. The maximum difference between the peak values of  $\sigma_r$  is less than 15% (from 1.14 to 1.32).
3. The difference in the stresses  $\sigma_\theta$  on the boundary in the critical area  $20^\circ < \theta < 80^\circ$  is less than 10%.
4. The shapes of the stress diagrams are essentially the same.



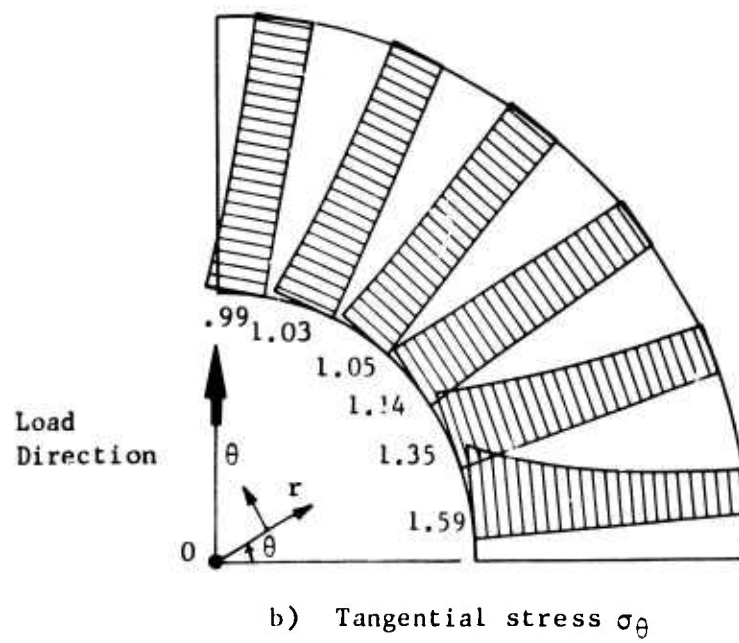
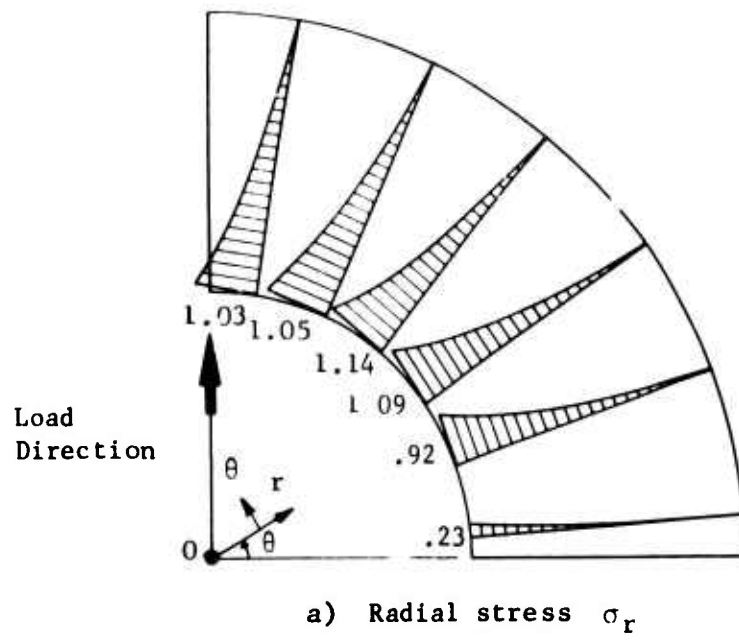
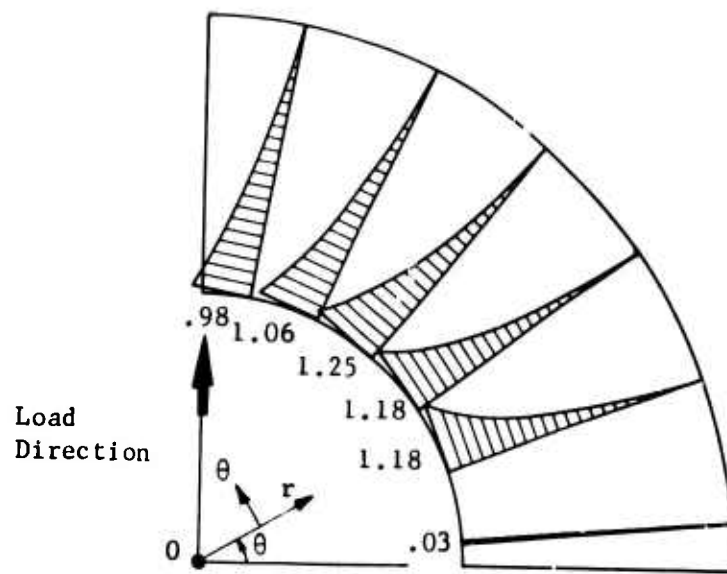
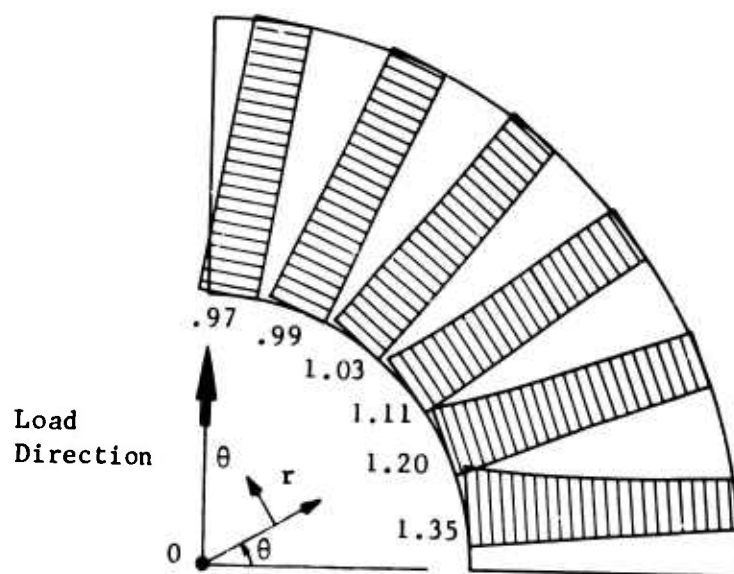


Figure 9. Stress Diagrams for Isotropic Material (Steel).

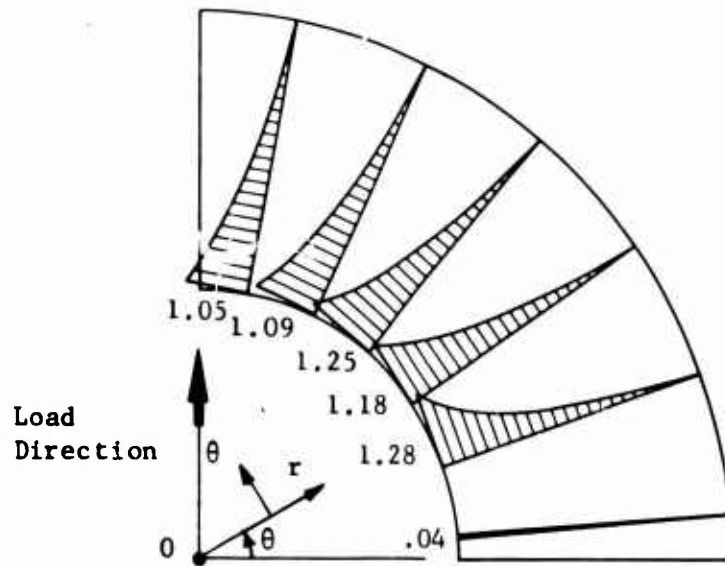


a) Radial stress  $\sigma_r$

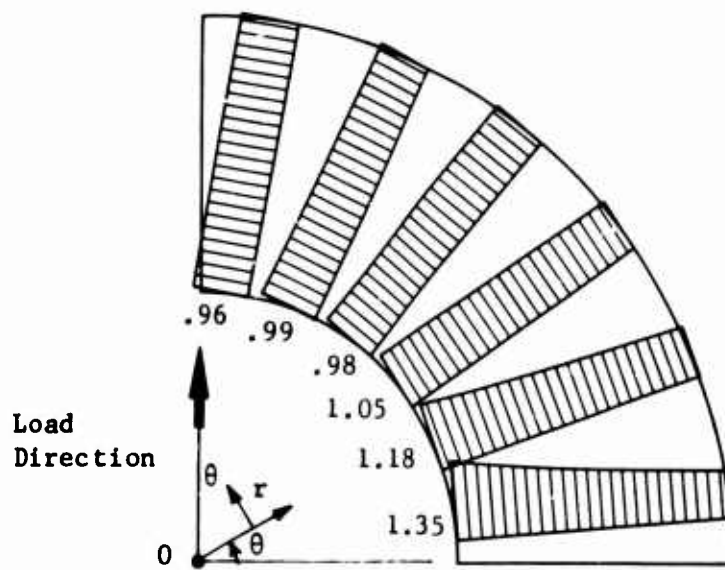


b) Tangential stress  $\sigma_\theta$

Figure 10. Stress Diagrams for Glass Composite Material ( $V_F = 0.6$ ).

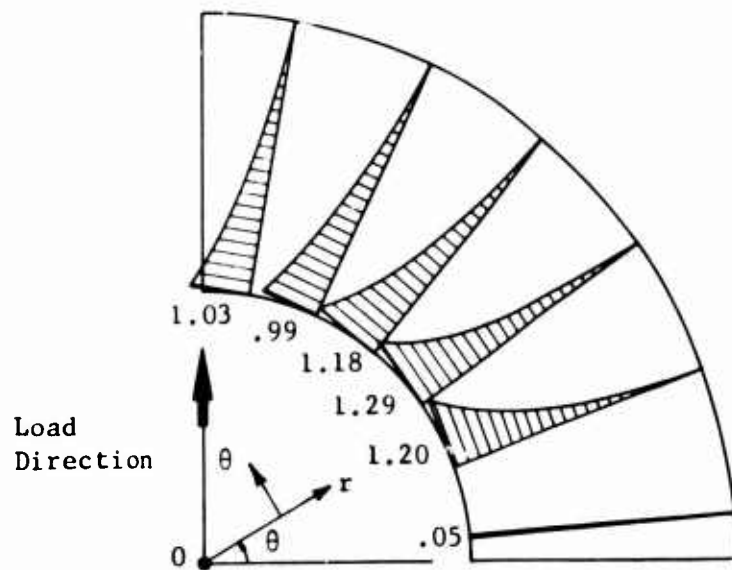


a) Radial stress  $\sigma_r$

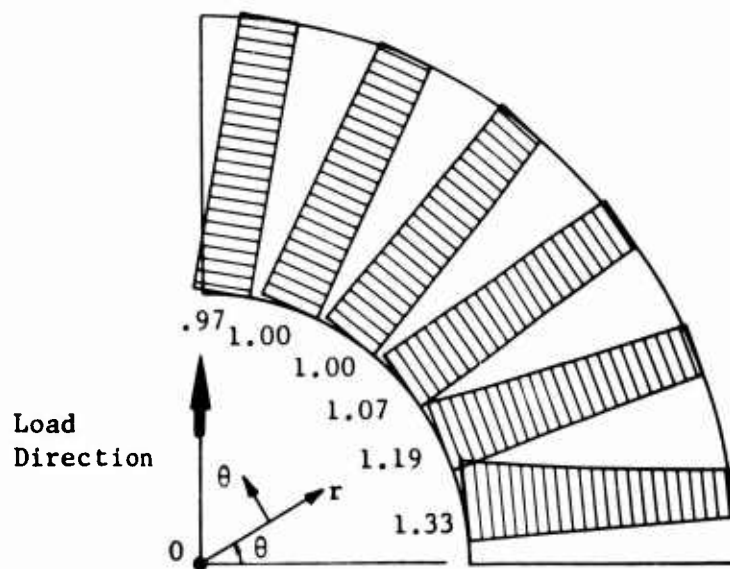


b) Tangential stress  $\sigma_\theta$

Figure 11. Stress Diagrams for Glass Composite Material ( $V_F = 0.7$ ).

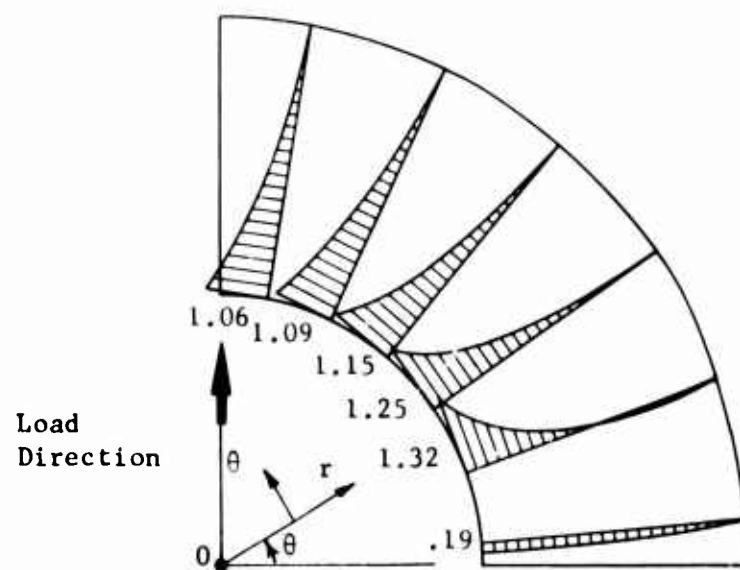


a) Radial stress  $\sigma_r$

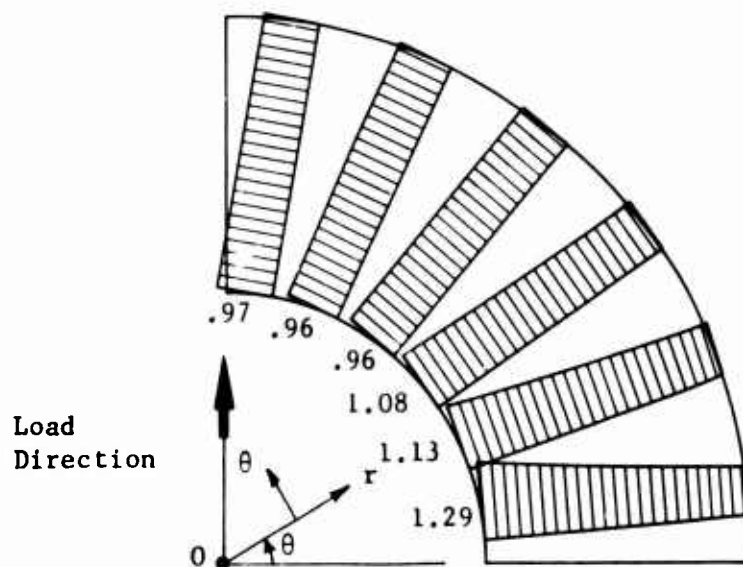


b) Tangential stress  $\sigma_\theta$

Figure 12. Stress Diagrams for Glass Composite Material ( $V_F = 0.8$ ).



a) Radial stress  $\sigma_r$



b) Tangential stress  $\sigma_\theta$

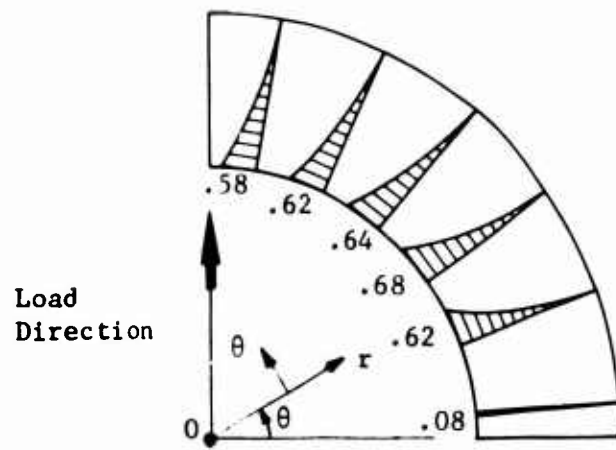
Figure 13. Stress Diagrams for Boron Composite Material ( $V_F = 0.7$ ).

It is therefore possible to analyze the stresses in this type of joint by assuming the isotropy of the material. The resultant stresses will, for design purposes, be sufficiently accurate.

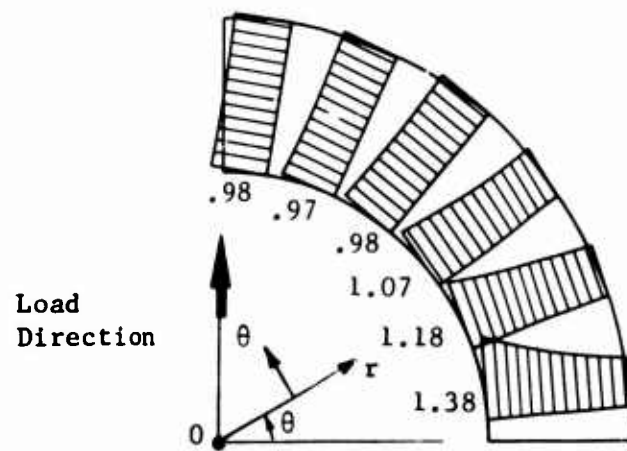
Another type of parametric study was performed in order to establish the variation of the stresses with respect to the changes of the ratio  $t/z$ . Two types of materials, isotropic and glass composite ( $V_F = 0.7$ ), were also considered. Table I gives the characteristics of the joints for which stresses are shown in Figures 14 through 19.

TABLE I. CHARACTERISTICS OF JOINTS				
Figure No.	Material	Ratio $\frac{t}{a}$	Maximum, $\sigma_r$	$\sigma_\theta$ where $\sigma_r$ is maximum
14	Isotropic	0.57	0.68	1.07
15	Glass composite	0.57	0.59	0.98
16	Isotropic	1.14	1.28	1.08
17	Glass composite	1.14	1.22	1.02
18	Isotropic	1.71	2.00	1.29
19	Glass composite	1.71	1.92	1.20

The results shown in this table confirm the conclusion of the preceding parametric study referring to the small influence of the material properties on the maximum stresses.



a) Radial stress  $\sigma_r$



b) Tangential stress  $\sigma_\theta$

Figure 14. Stress Diagrams for Isotropic Material (Steel).

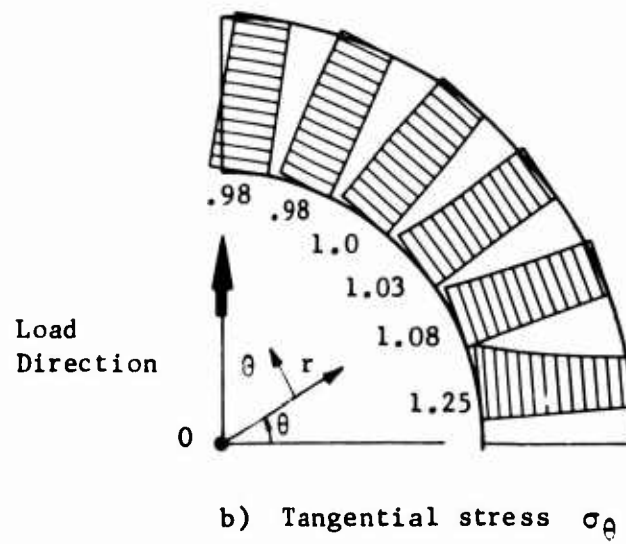
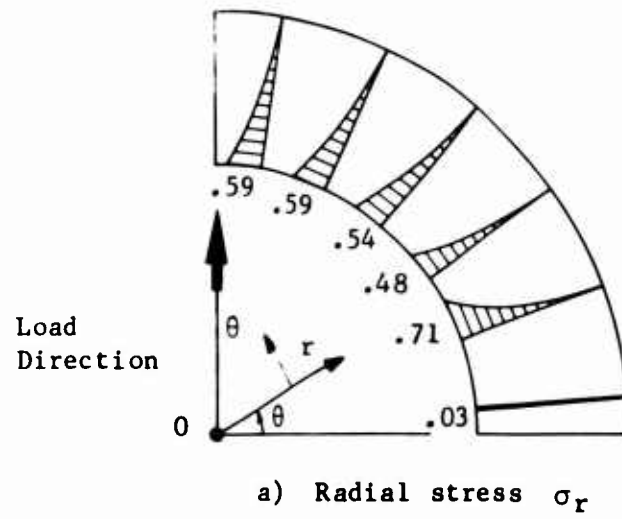
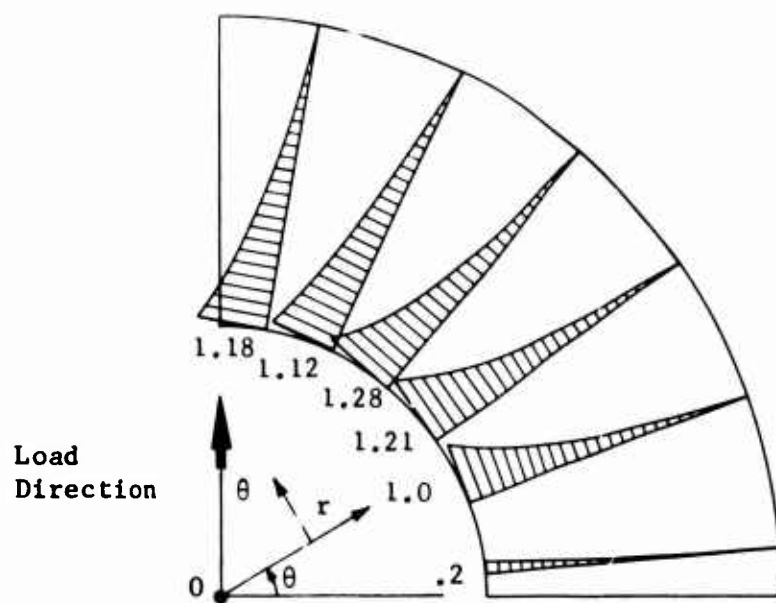
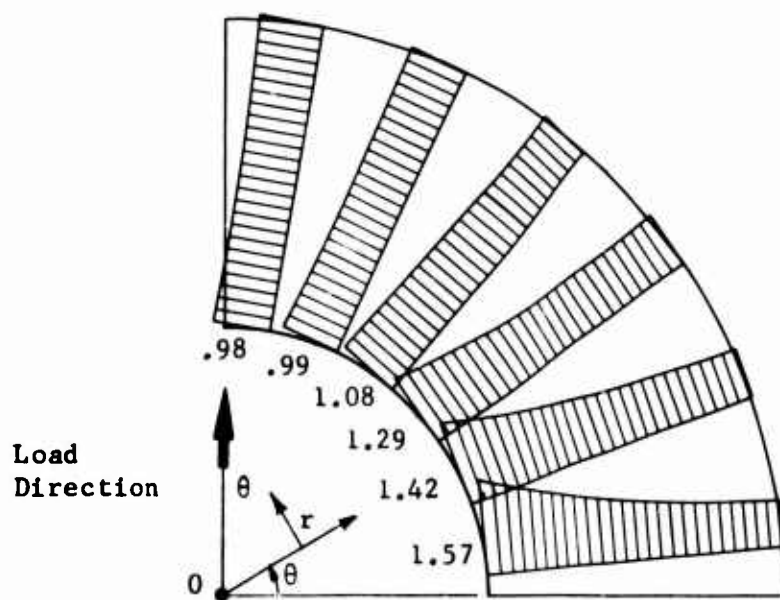


Figure 15. Stress Diagrams for Glass Composite Material ( $V_F = 0.7$ ).





a) Radial stress  $\sigma_r$



b) Tangential stress  $\sigma_\theta$

Figure 16. Stress Diagrams for Isotropic Material (Steel).

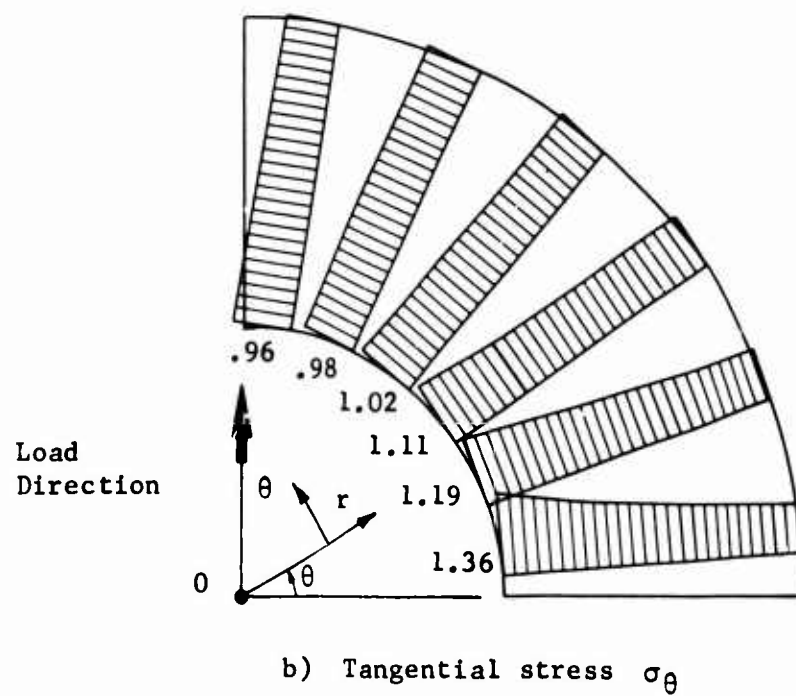
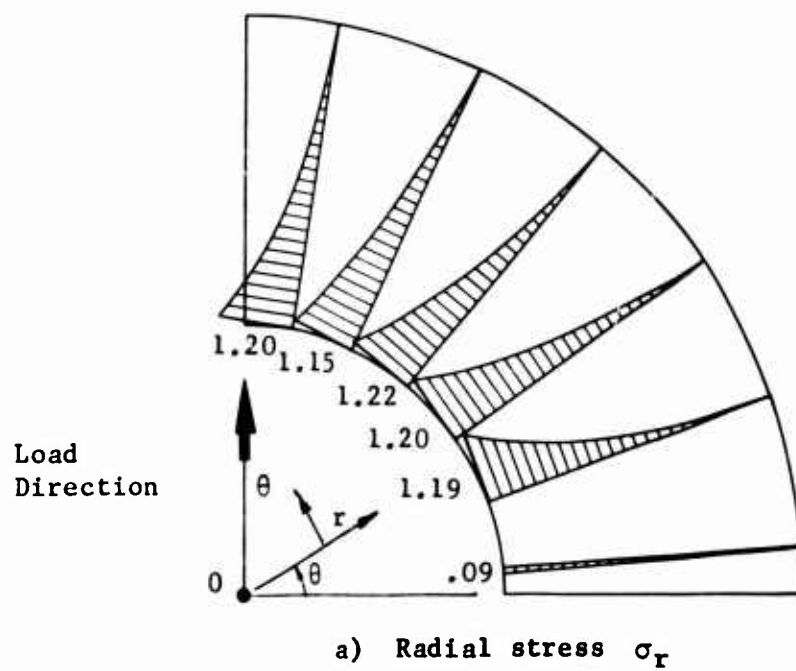
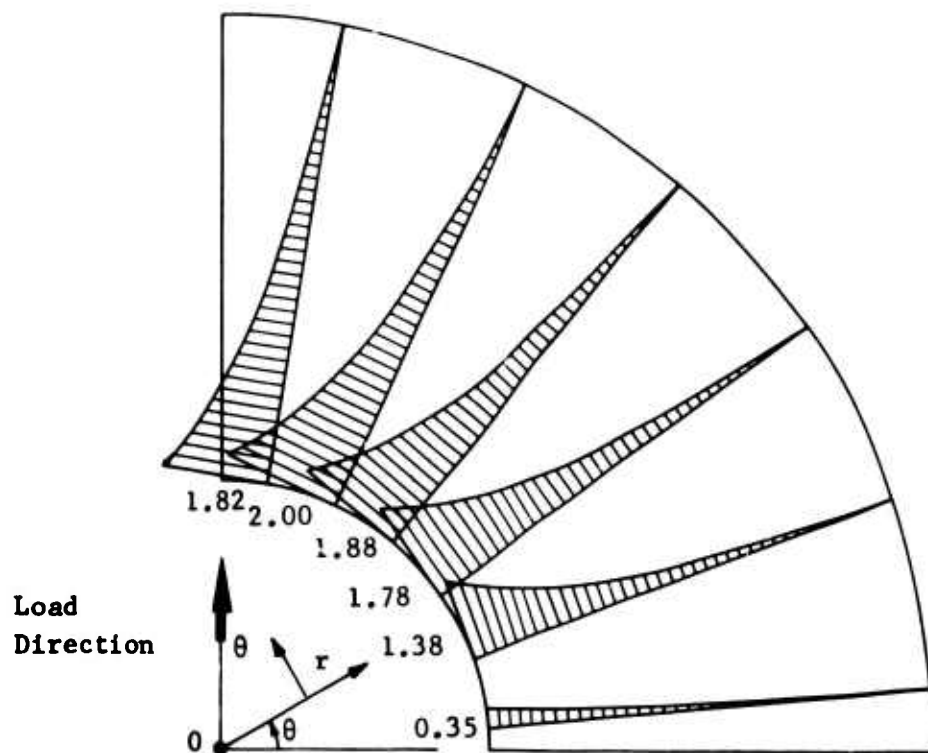
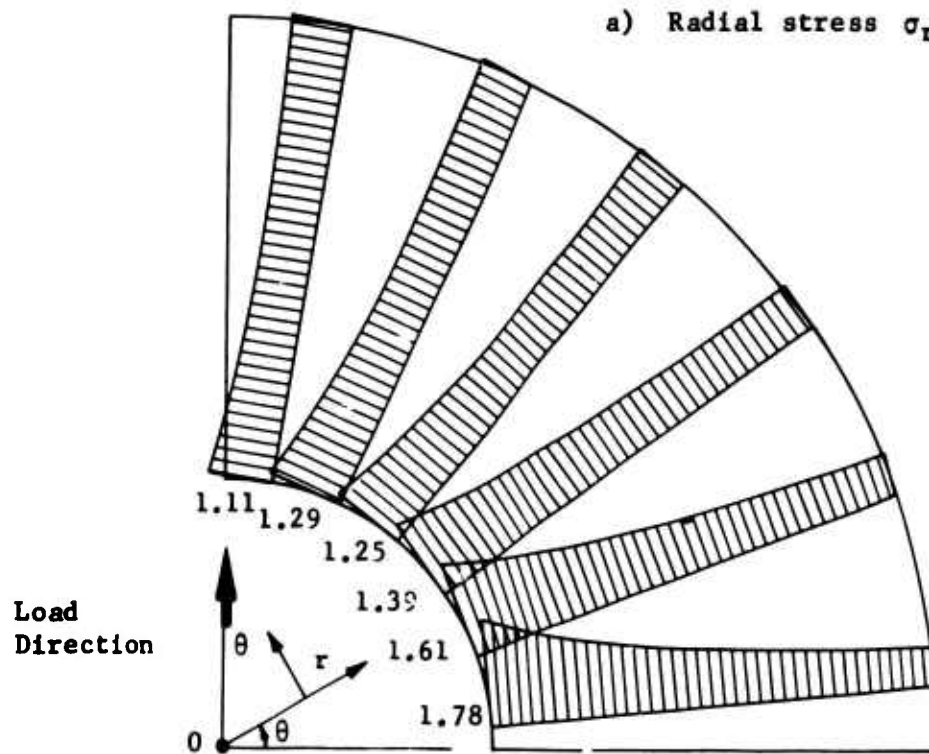


Figure 17. Stress Diagrams for Glass Composite Material ( $V_F = 0.7$ ).



a) Radial stress  $\sigma_r$



b) Tangential stress  $\sigma_\theta$

Figure 18. Stress Diagrams for Isotropic Material (Steel).

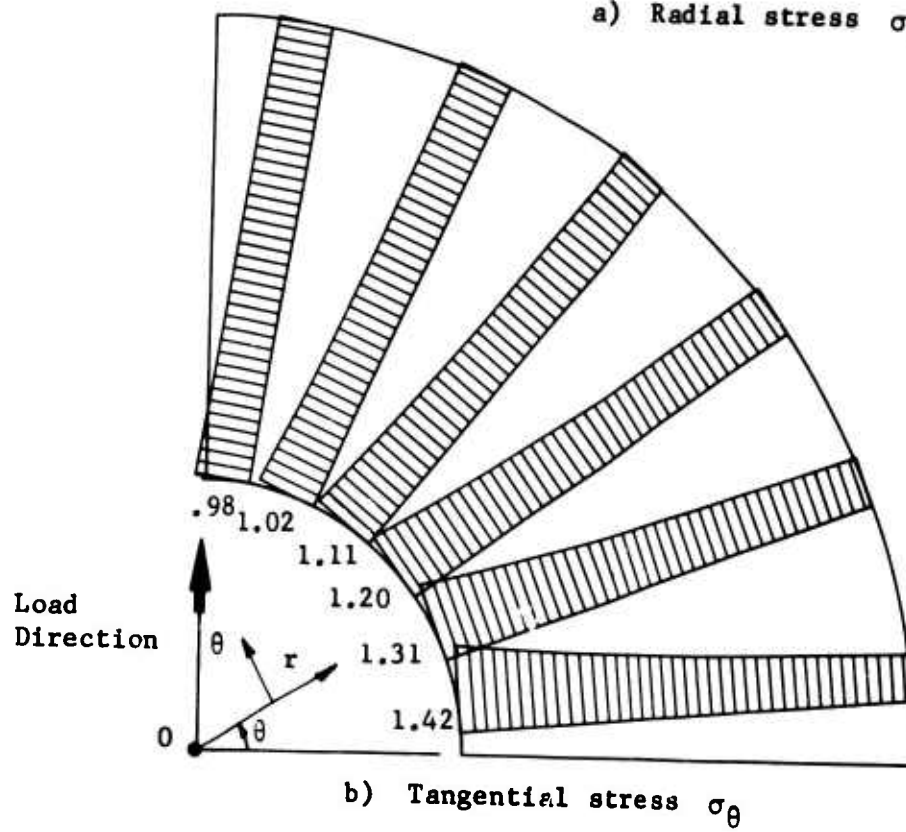
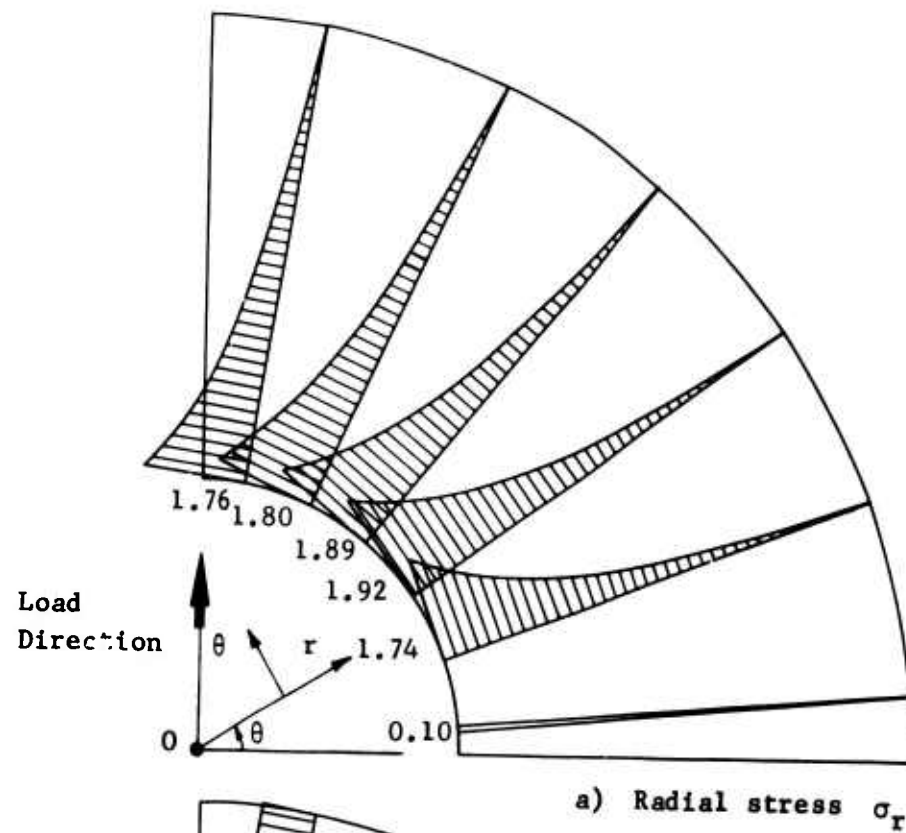


Figure 19. Stress Diagrams for Glass Composite Material ( $V_F = 0.7$ ).

Figure 20 is a plot of the ratio  $\sigma_r(\max)/\sigma_{avg}$  against  $t/a$ ,  $\sigma_{avg}$  being the average stress  $P/2t$  and  $P$  the total applied tensile load.

This relation can be expressed with minimal error by the linear formula

$$\sigma_r(\max) = 1.13 \sigma_{avg} \frac{t}{a} \quad (54)$$

Figure 21 shows the curve of  $\sigma_\theta/\sigma_{avg}$  with  $t/a$  as the independent variable. The value  $\sigma_\theta$  is corresponding to the point in which  $\sigma_r$  is a maximum. This point is where the factor of safety is minimum, as we will see later. The curve of Figure 21 can be approximately expressed by the following:

$$\sigma_\theta = \left[ 1 + 0.079 \left( \frac{t}{a} \right)^2 \right] \sigma_{avg} \quad (55)$$

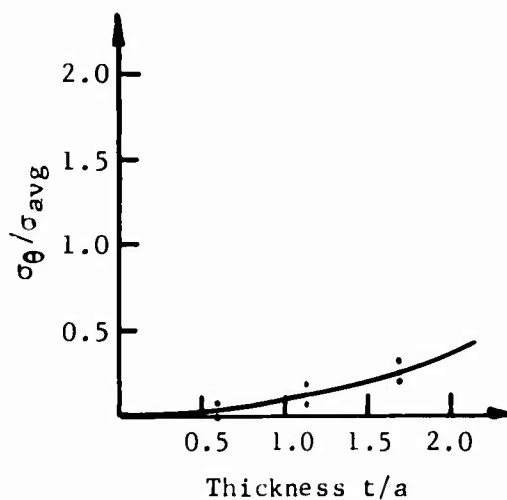
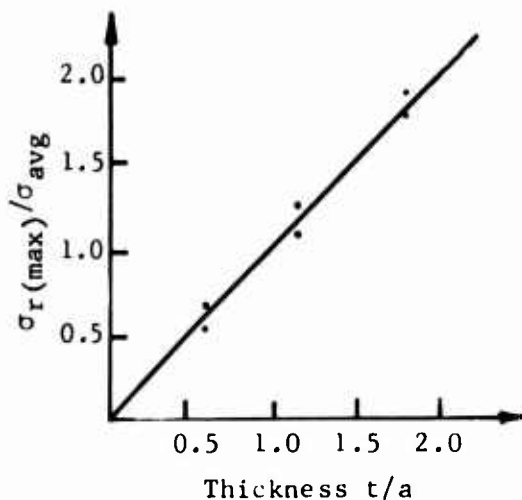


Figure 20. Maximum Radial Stress.

Figure 21. Value of  $\sigma_\theta$  Where  $\sigma_r$  Is Maximum.

## STRUCTURAL DESIGN OF BOLT JOINTS

### Failure Criteria

Once the stress distribution is known, it is necessary to adopt a failure criterion in order to establish the factor of safety of the joint. A failure theory widely used is the so-called interaction formula<sup>9</sup>

$$\frac{\sigma_x}{\sigma_x^*}^2 - \frac{\sigma_x \sigma_y}{\sigma_x^* \sigma_y^*} + \frac{\sigma_y}{\sigma_y^*}^2 + \frac{\tau_{xy}}{\tau_{xy}^*}^2 = 1 \quad (56)$$

where  $\sigma_x$ ,  $\sigma_y$ , and  $\tau_{xy}$  are the macromechanical stresses and  $\sigma_x^*$ ,  $\sigma_y^*$ , and  $\tau_{xy}^*$  are the failure stresses. The normal stresses  $\sigma_x$  and  $\sigma_y$  are positive (negative) corresponding to tension (compression). The failure stress  $\sigma_x^*$  must be the failure stress in tension if  $\sigma_x$  is positive, and the failure stress in compression if  $\sigma_x$  is negative. The same criterion is used with respect to  $\sigma_y$  and  $\sigma_y^*$ . In the formula,  $\sigma_x^*$  and  $\sigma_y^*$  must always be positive. The interaction formula as a failure criterion was introduced in Reference 7. Other failure criteria are presented in References 11 and 12.

In the following, we will adopt the interaction formula (56) as the representative failure criterion. By referring equation (56) to the polar coordinates  $r$  and  $\theta$ , and by taking into account that  $\tau_{r\theta} = 0$  for the critical area of the joint, we get the factor of safety  $\mu$ :

$$\mu = \frac{1}{\sqrt{\left(\frac{\sigma_r}{\sigma_r^*}\right)^2 - \frac{\sigma_r \sigma_\theta}{\sigma_r^* \sigma_\theta^*} + \left(\frac{\sigma_\theta}{\sigma_\theta^*}\right)^2}} \quad (57)$$

If the joint is made in the manner indicated in Figure 22, the stress  $\sigma_r^*$  is the tension failure stress for the unidirectional composite in the fiber direction, and  $\sigma_\theta^*$  is the compression failure stress in the normal direction.

#### Design Procedure

The design procedure for a bolt joint will be illustrated on a joint selected from the Mohawk plane of Grumman Aircraft Company. This joint corresponds to the wing-fuselage connection, and the ultimate load in tension is  $P = 30,000$  lb. Information about the joint design is given in References 13, 14, and 15.

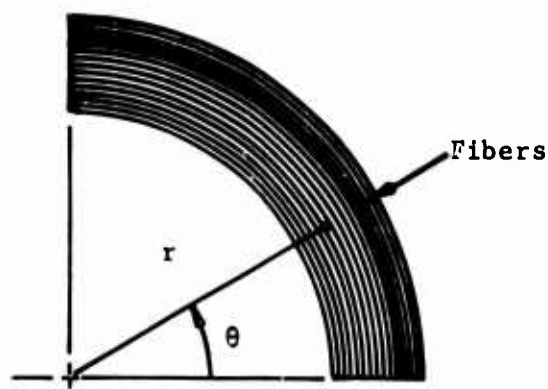


Figure 22. Unidirectional Composite Joint.

Figure 23 shows the preliminary adopted dimensions by using glass fibers and epoxy resin with a fiber volume content of 70% to 75%.

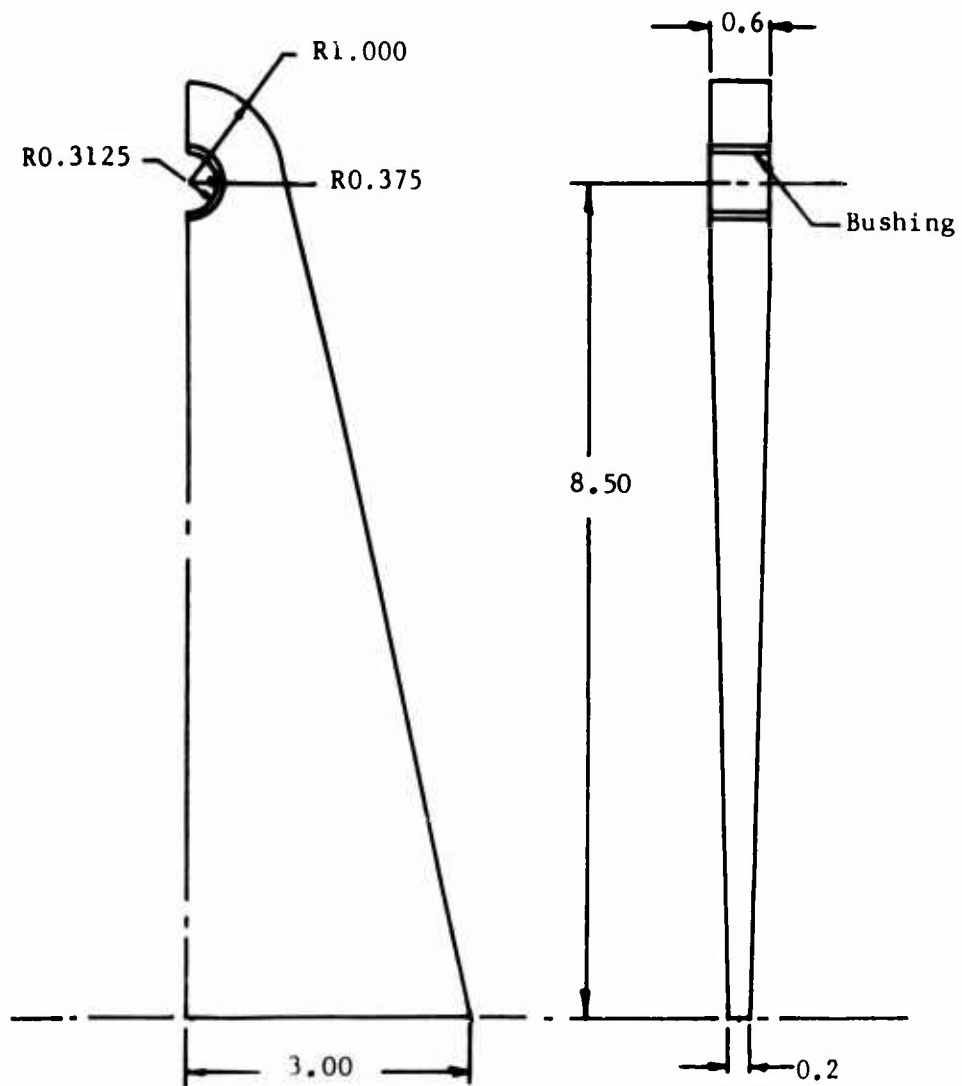


Figure 23. Dimensions of the Bolt Joint (inch).

The ratios  $t/a$  and  $\sigma_{avg}$  are

$$\frac{t}{a} = \frac{1.000 - 0.375}{0.375} = 1.67$$

$$\sigma_{avg} = \frac{30,000}{0.6 \cdot 0.625 \cdot 2} = 40,000 \text{ psi}$$

Then, from equations (54) and (55), we get

$$(\sigma_r)_{max} = 1.89 \sigma_{avg} \quad , \quad \sigma_{\theta} = 1.22 \sigma_{avg}$$

or, with the value of  $\sigma_{avg}$  obtained before,

$$(\sigma_r)_{max} = 75,600 \text{ psi} \quad , \quad \sigma_{\theta} = 48,800 \text{ psi}$$

The failure stresses for the unidirectional composite employed are

$$\sigma_r^* = 30,000 \text{ psi} \quad , \quad \sigma_{\theta}^* = 225,000 \text{ psi}$$

The failure stress in compression corresponds to the 0.2% limit of strain. It is not an actual 'failure stress.' By using this value and on the basis of equation (57), a factor of safety of  $\mu = 0.38$  results and the failure load will be

$$P^* = 0.38 \cdot 30,000 = 11,400 \text{ lb}$$

However, the failure load obtained from tests is about 26,000 lb, as shown in the discussion of experimental results in this report. This discrepancy between analytical and experimental results stems from the following fact: When the load reaches a value of 12,000 lb, a plastic area appears as shown in Figure 24, and the load-elongation diagram presents "jumps" which

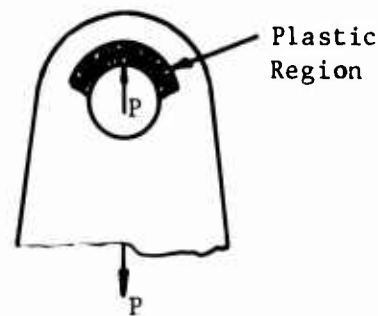


Figure 24. Plastic Region.



indicate the successive progress of the plasticized area. Thus, the load obtained from the linear stress analysis only indicates the beginning of the formation of the plastic areas.

By taking bushings of larger external diameter, we can make joints of smaller ratio  $t/a$ . The following two values are considered:

$$\frac{t}{a} = 1.0 \quad , \quad \frac{t}{a} = 0.66$$

The corresponding values of  $\mu$  obtained by following the same procedure as above result in the values in Table II.

TABLE II. SAFETY FACTORS		
$\frac{t}{a}$	1.0	0.66
$\sigma_{avg}$	50,000	62,500
$(\sigma_r)_{max}$	50,500	46,600
$\sigma_\theta$	53,900	64,700
$\mu$	0.504	0.582

This shows that the factor of safety increases when  $t/a$  decreases. However, in practical applications of joints, other limitations exist that do not permit the use of a very low value of  $t/a$ . A ratio  $t/a = 1$  appears to be quite adequate for most of the bolt joints of composite materials.

The ultimate failure load, which is the maximum load that can possibly be applied to the joint before it breaks completely, is approximately double the load given by the described procedure. This assumption proved to be valid only for  $0.9 \leq t/a \leq 1.1$ .

#### EXPERIMENTAL RESULTS

The experimental program was divided into two parts, one concerning a simple loop joint and the other the bolt joint.

### Loop Joint

An experimental program concerned with the stress distribution and failure loads of loop joints was developed.

The joints tested to failure had the dimensions indicated in Figure 25.

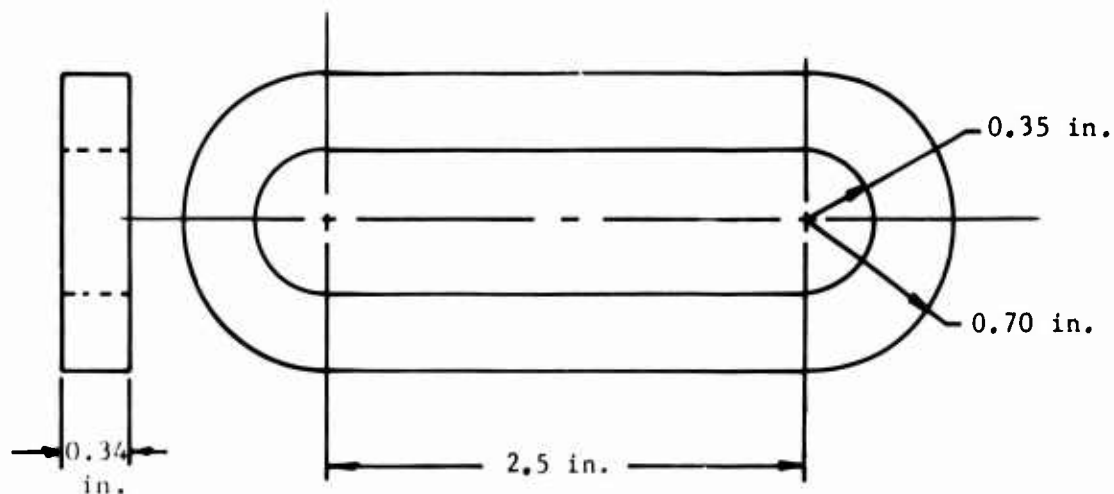


Figure 25. Dimensions of the Loop Joint.

The specimens were made of glass composite material and were tested in tension. The load was applied by using a fixture schematically shown in Figure 26. A bushing was not used between bolt and joint, and special care was taken to ensure perfect initial contact between bolt and joint at the surface of load transference.

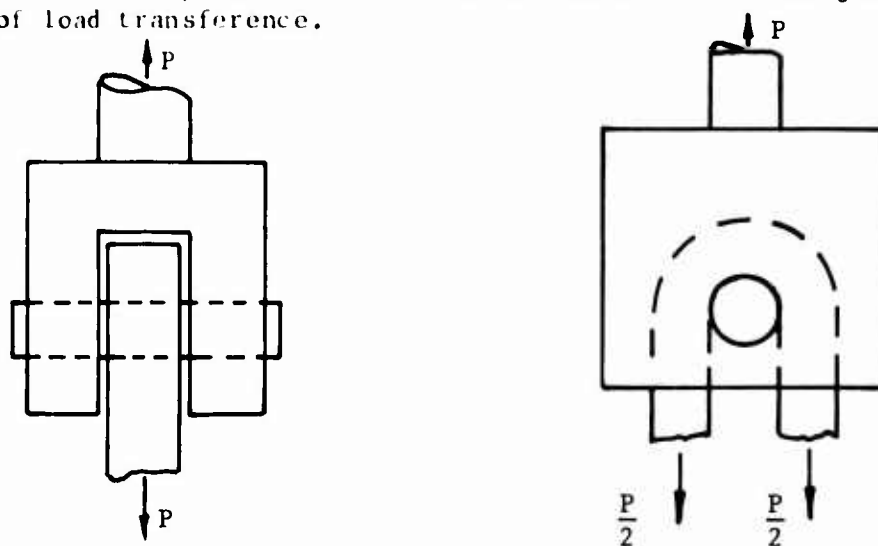


Figure 26. Attachment Device for Testing.

The failure loads obtained from the tests are summarized in Table III.

TABLE III. FAILURE LOADS			
Specimen No.	Dimensions of a Simple Cross Section, in.	Failure load, lb	Avg Failure Stress, $\text{psi} \times 10^{-3}$
1	0.342 x 0.308	12,900	61.5
2	0.346 x 0.295	11,250	55.2
3	0.344 x 0.282	10,400	53.7
4	0.348 x 0.301	13,350	63.2
5	0.346 x 0.378	13,100	52.0

The ultimate strength of the joint's material in simple tension was 150-180 kpsi. Therefore, the efficiency of the loop joint is about 0.33. In other words, the ultimate strength of the loop joint is one-third of the strength of the same material in simple tension. This conclusion is limited only to joints with a ratio  $t/a = 1$ .

Figures 27, 28, and 29 illustrate the location and form of failure. The regions of resin under failure (opaque areas in the photographs) encompass three-fourths of the total section. It is also possible to observe that the failure is produced for  $20^\circ < \theta < 80^\circ$ .

A failure test was performed by using a plastic homogeneous, brittle material loop joint. Figure 30 shows the failure produced.

Photoelastic studies were conducted to establish the stress distribution in both orthotropic and isotropic loop joints. Figure 31 shows the isoclines for  $0^\circ$ ,  $30^\circ$ , and  $60^\circ$  for the isotropic joint. Similar patterns were obtained on orthotropic loop joints by using the reflection method of photoelasticity. The position of the inclines confirms the analytical results in the sense that the shear stresses  $\tau_{rg}$  are very small. In fact, the isocline lines are approximately radial straight lines, forming angles of  $0^\circ$ ,  $30^\circ$ , and  $60^\circ$ , respectively.

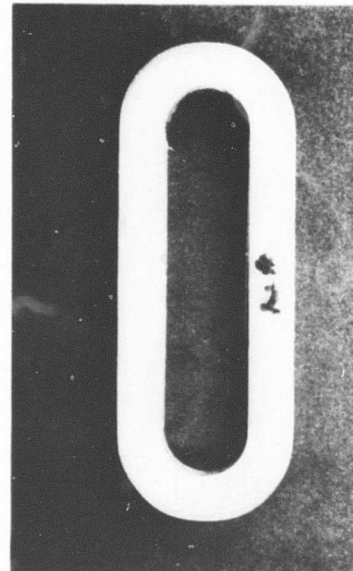


Figure 27. Loop Joint of Composite Material After Testing to Failure.

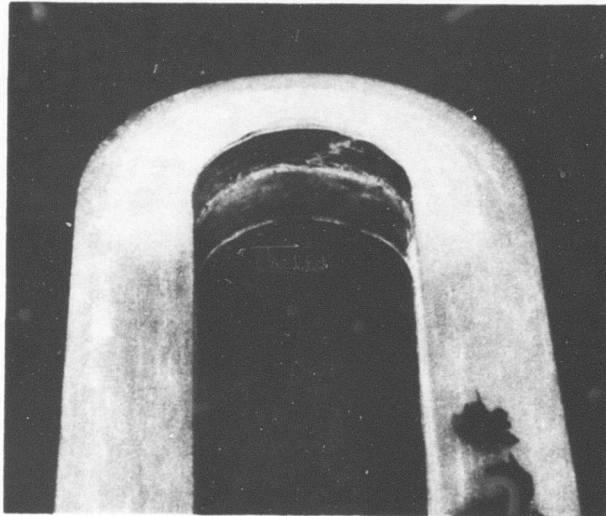


Figure 28. Loop Joint of Composite Material After Testing to Failure.

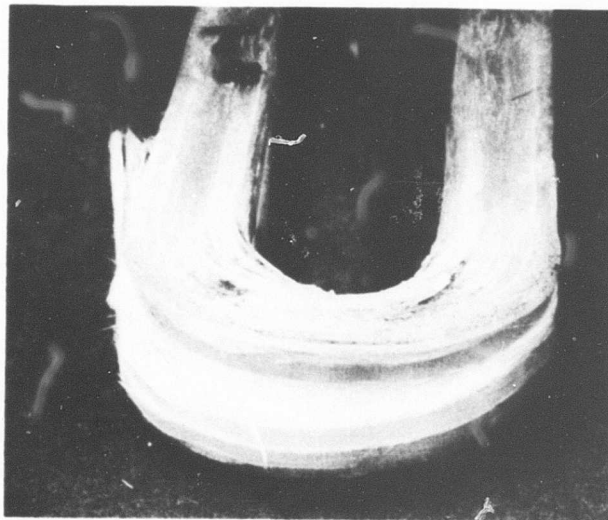


Figure 29. Loop Joint of Composite Material After Testing to Failure.

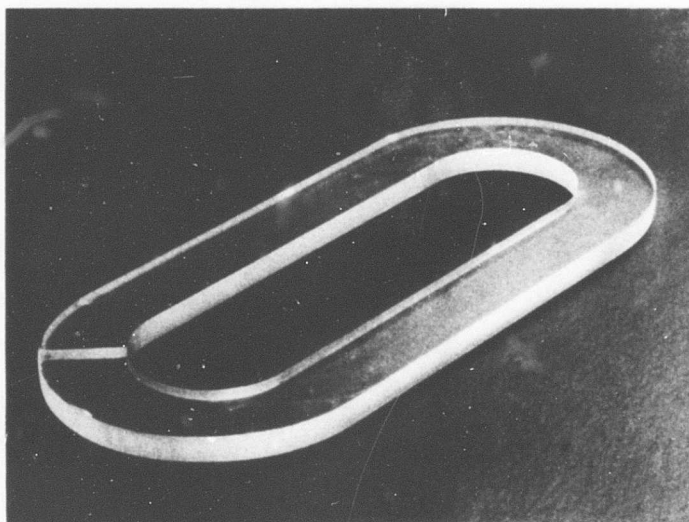


Figure 30. Failure of an Isotropic Joint of Brittle Material.



(a) Isoclines of  $0^\circ$

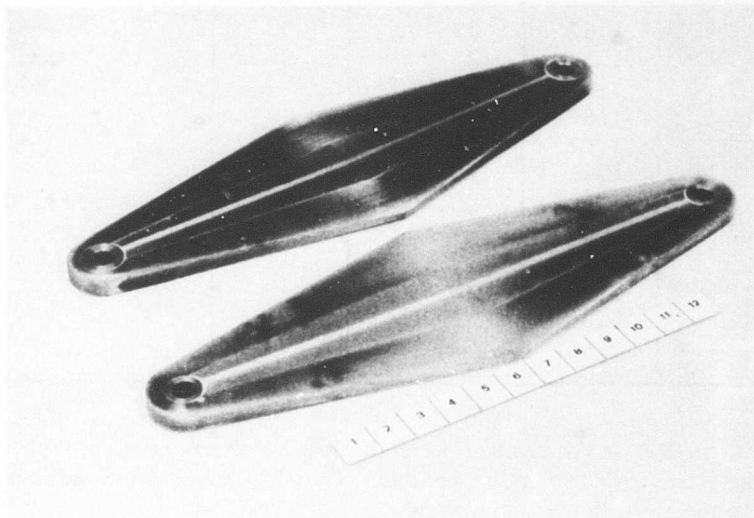
(b) Isoclines of  $30^\circ$

(c) Isoclines of  $60^\circ$

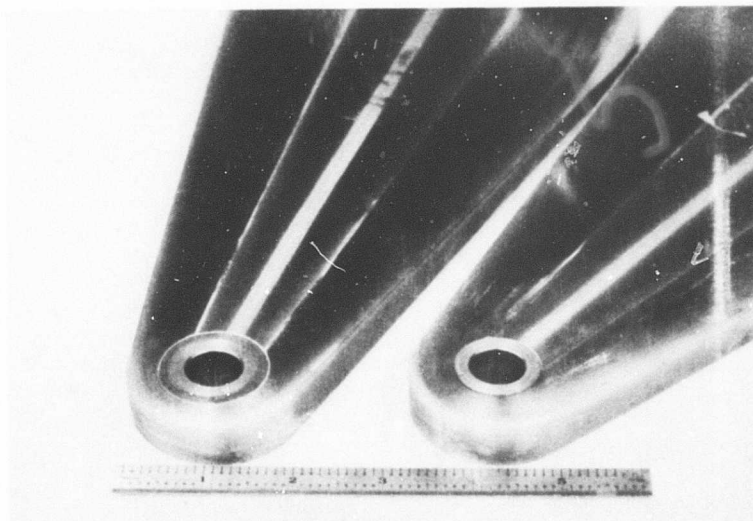
Figure 31. Isoclines for  $0^\circ$ ,  $30^\circ$ , and  $60^\circ$  for the Isotropic Joint.

### Bolt Joint

The tested bolt joints were similar to those shown in Figure 32. Table IV indicates the characteristics and ultimate load of the tested bolt joints.



(a) Composite Joints



(b) Composite Joints

Figure 32. Bolt Joints Similar to Those Tested.

TABLE IV. SPECIMEN CHARACTERISTICS AND ULTIMATE LOADS				
Specimen No.	Ratio t/a	Ultimate Load, lb	Figure No.	Observations
1	1.67	29,700	33	--
2	1.67	25,000	34	--
3	1.67	24,500	--	--
4	1.67	28,100	--	--
5	1.67	26,700	--	--
6	1.67	25,900	--	--
7	1.00	45,400	35	Laterally Supported
8	1.00	44,500	--	Laterally Supported
9	1.00	39,400	36	--
10	1.00	38,800	--	--
11	0.66	42,200	37	--
12	0.66	41,500	--	--

Specimens 7 and 8 were tested with a device that restricted lateral expansion of the joint under high loads. In this way, an increment of the ultimate load of 10% was obtained.

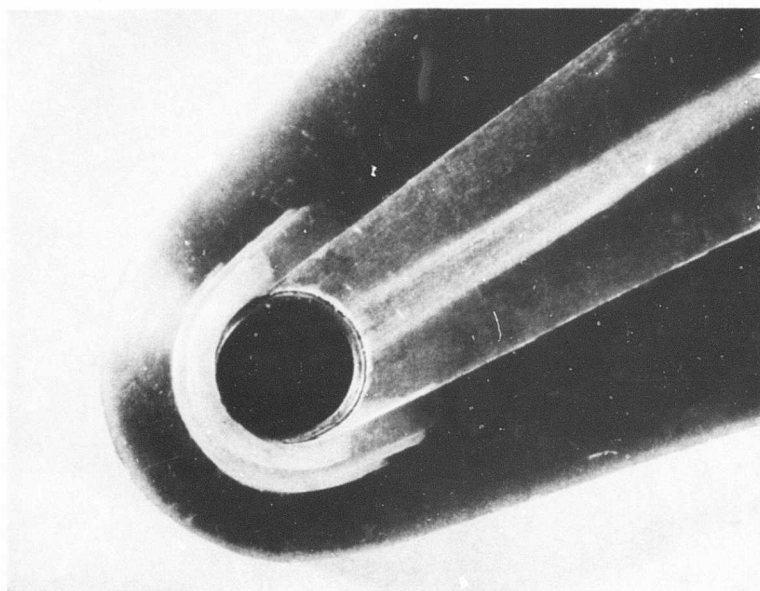


Figure 33. Composite Joint After Failure.

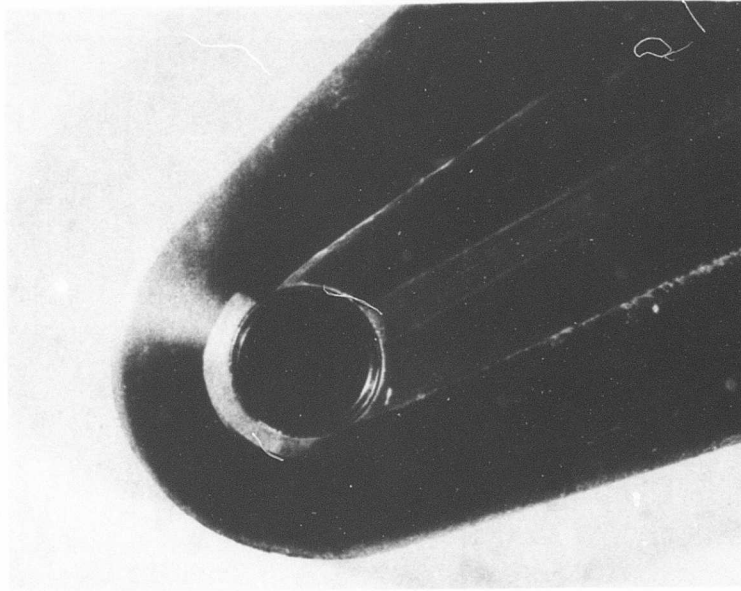


Figure 34. Composite Joint After Failure.

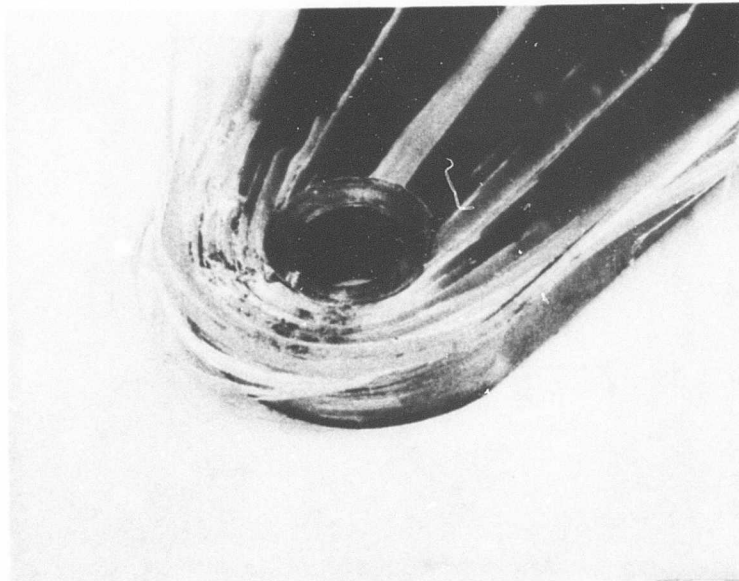


Figure 35. Composite Joint After Failure.



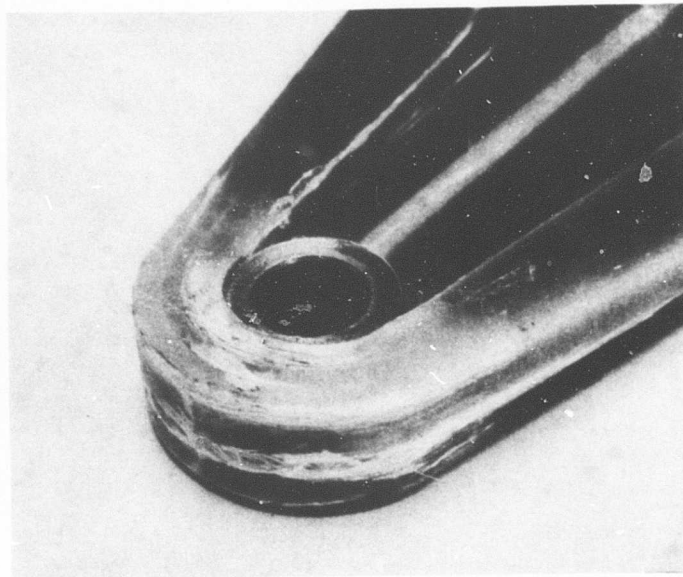


Figure 36. Composite Joint After Failure.

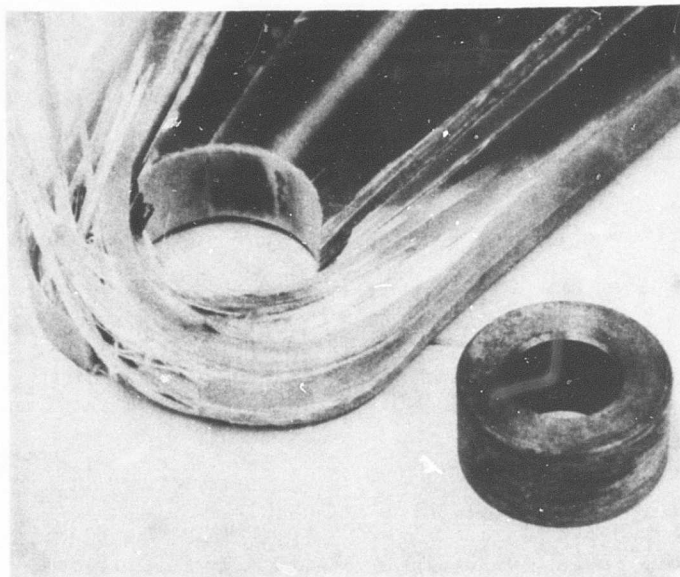


Figure 37. Composite Joint After Failure.

Figure 38 represents a typical load-displacement curve obtained during the tension test of the bolt joints. It can be observed that there are several "jumps" which correspond to several steps of failure that occurred during the test. The ultimate load shown in Table IV is the highest, after which the joint breaks.

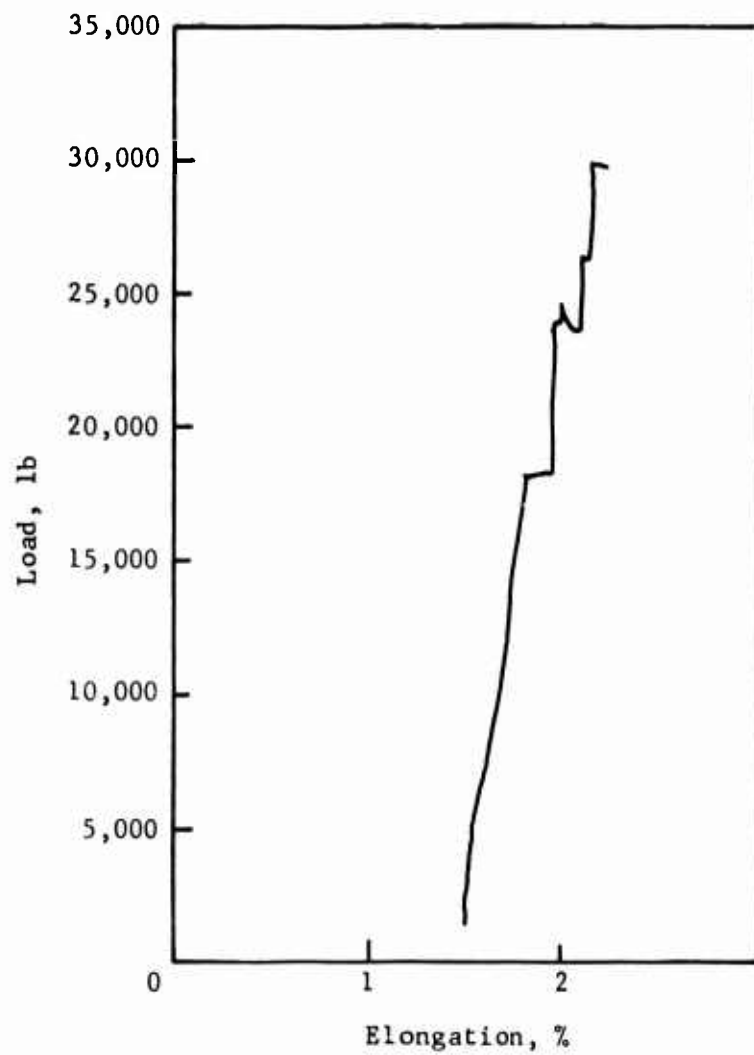


Figure 38. Testing Machine Load-Displacement Plotted.

Figure 39 shows the specimen mounted in the testing machine for tension testing.

#### FABRICATION PROCESS

The following procedure was adhered to in the fabrication of test specimens for this program.

#### Material

1. 12 End Roving 5994 Glass
2. Resin System
  - a. Epon 828 50 phr
  - b. Epon 1031 50 phr
  - c. NMA (methyl endomethylene tetrahydro phthalic anhydride catalyst) 90 phr
  - d. Benzyldimethylamine (BDHA accelerator) 0.50 phr

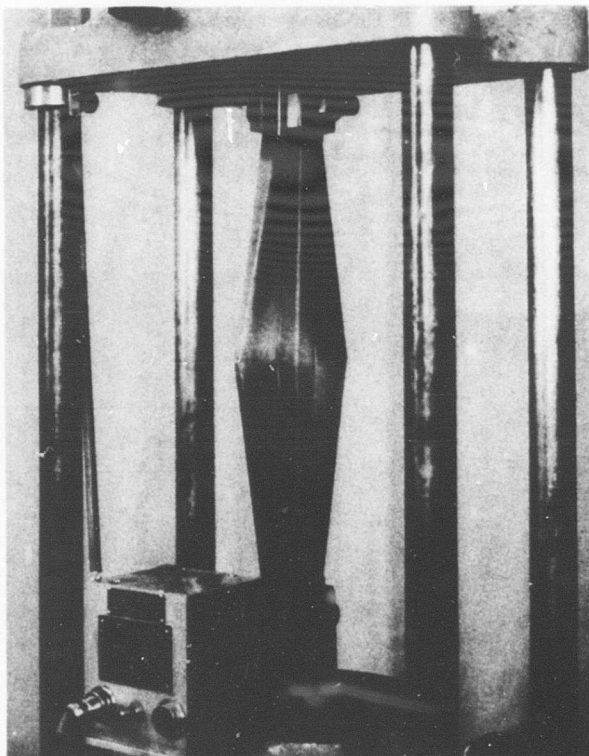


Figure 39. Specimen Mounted in the Test Machine.

#### Tooling

Winding Tool (Figure 40), Aluminum-Constructed Material

#### Equipment

1. Temperature Controlled Hot-Melt Coater
2. Fiber Tensioning Device
3. Winding Machine (special throat-deep; see Figure 41)
  - a. Traverse-Controlled
  - b. Revolution Infinite Control

#### Winding Procedure

1. Set winding traverse to 0.600 in. to yield 9 revolutions per layer.
2. Set fiber tensioning device at 2-pound fiber tension.
3. Regulate hot-melt coater temperature to 170°-180°F.
4. Preheat winding fixture to 160-180°F.

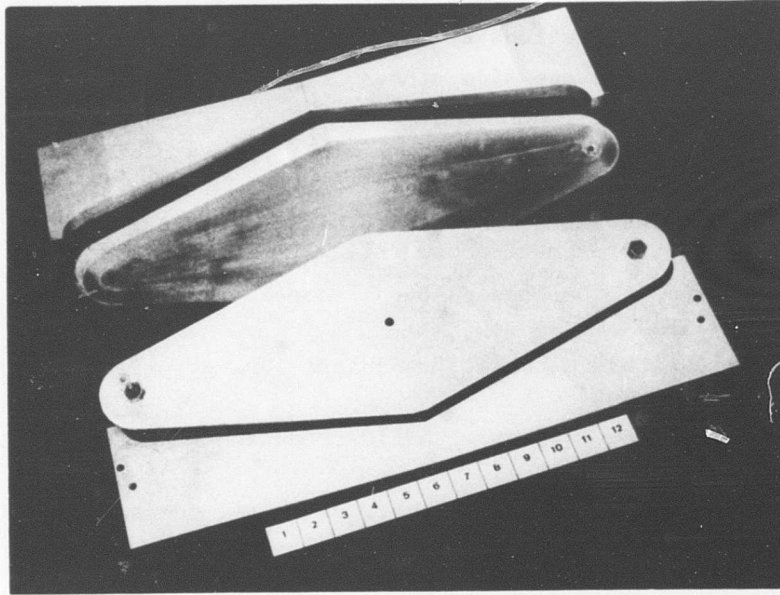


Figure 40. Aluminum-Constructed Winding Tool.

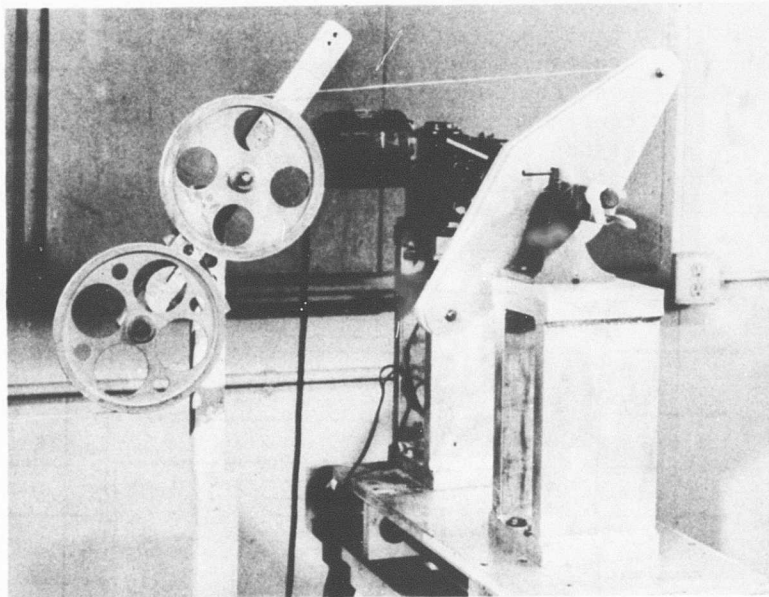


Figure 41. Winding Machine.

5. Wind 568 revolutions at 2-pound fiber tension.
6. Apply first-stage side plates; close side plates to within 0.062 in. to stops.
7. Place assembly in a 225°F regulated press platen; close side plates just short of resin gelation.
8. Raise temperature to 250°F, reside for 1 hour at 250°F, and cure at 325°F for 1 hour. Allow to cool under pressure.
9. Disassemble tool and remove precured first-stage laminate from tool.
10. Remove excess material; sandblast faying surface for second-stage winding.
11. Wind 500 revolutions with fiber tension reduced to 1.5 pounds. Total winding revolutions of 1035.
12. Repeat operations 6 through 9 using second-stage side plates.
13. Remove part from tool and report all physical properties.

#### STRESS CONCENTRATION AROUND CIRCULAR HOLES IN ORTHOTROPIC PLATES

##### General Formulas

The technical importance of the stress concentration around holes is well known. The concentration factors have been established for the most important cases that appear for isotropic materials in technical applications. For example, in a tensioned plate with a circular hole, a diagram of stresses  $\sigma_x$  appears as shown in Figure 42. It is possible to see that the stress concentration factor is 3, because the peak of stress is three times larger than that of the applied stress.

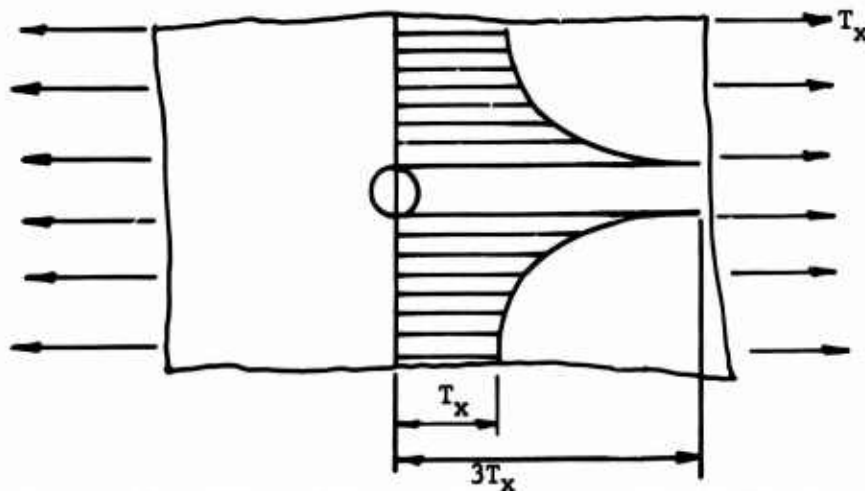


Figure 42. Stress Concentration in Isotropic Sheet.

For the correct use of structural composite materials, it is important to know the influence of the relative volume and directions of the fibers on the concentration factors. This is of fundamental importance in the case of a structure with alternative loads, because the maximum stress concentration points are the origin of the failure lines.

In this part of the report, we will develop the analytical elements and a computer program that will permit us to find the concentration factors in an orthotropic plate with a circular hole. As external loads, we consider tension and pure shear. By combining the results of these two cases, it is possible to solve almost all states of stresses, assuming that the hole's diameter is small with respect to the dimensions on the middle plane of the plate.

The theoretical elements of the complex solution of the plane state of stresses are taken from Reference 16.

The fundamental differential equation of an orthotropic plate in plane stress is

$$b_{11} \frac{\partial^4 \phi}{\partial y^4} + (2b_{12} + b_{33}) \frac{\partial^4 \phi}{\partial x^2 \partial y^2} + b_{22} \frac{\partial^4 \phi}{\partial x^4} = 0 \quad (58)$$

where  $b_{11}$ ,  $b_{12}$ ,  $b_{22}$ , and  $b_{33}$  are the elements of the matrix that connects stresses and strains; i.e.,

$$\begin{Bmatrix} \epsilon_x \\ \epsilon_y \\ \gamma_{xy} \end{Bmatrix} = \begin{bmatrix} b_{11} & b_{12} & 0 \\ b_{12} & b_{22} & 0 \\ 0 & 0 & b_{33} \end{bmatrix} \begin{Bmatrix} \sigma_x \\ \sigma_y \\ \tau_{xy} \end{Bmatrix}$$

It must be noted that the matrix  $b_{ij}$  is the inverse of matrix  $[C]$  mentioned earlier in this report, if this matrix also belongs to an orthotropic plate.

It is possible to rewrite this equation in the following form:

$$\left( \frac{\partial^2}{\partial x^2} + \alpha_1 \frac{\partial^2}{\partial y^2} \right) \left( \frac{\partial^2}{\partial x^2} + \alpha_2 \frac{\partial^2}{\partial y^2} \right) \phi = 0 \quad (59)$$

with the constants  $\alpha_1$  and  $\alpha_2$  defined by

$$\alpha_1 \alpha_2 = \frac{b_{11}}{b_{22}}, \quad \alpha_1 + \alpha_2 = \frac{2b_{12} + b_{33}}{b_{22}} \quad (60)$$

In other words,  $\alpha_1$  and  $\alpha_2$  are the roots of the equation in  $\alpha$

$$b_{22} \alpha^2 - (2b_{12} + b_{33}) \alpha + b_{11} = 0 \quad (61)$$

It is possible to demonstrate by energy considerations that the roots of equation (61) are real and positive if the coefficients  $b_{11}$ ,  $b_{12}$ ,  $b_{22}$ , and  $b_{33}$  correspond to any orthotropic material.

Taking the complex variable  $z = x + iy$  and its conjugate  $\bar{z} = x - iy$ , we put the stress function in the following form:

$$\Phi = f(z + \gamma_1 \bar{z}) + g(z + \gamma_2 \bar{z}) \quad (62)$$

Substituting this function into the differential equation (58), we can establish that the real part of equation (62) is a solution if  $\gamma_1$  and  $\gamma_2$  are given by

$$\gamma_1 = \frac{\sqrt{\alpha_1} - 1}{\sqrt{\alpha_1} + 1}, \quad \gamma_2 = \frac{\sqrt{\alpha_2} - 1}{\sqrt{\alpha_2} + 1} \quad (63)$$

Because  $\alpha_1$  and  $\alpha_2$  are real and positive,  $\gamma_1$  and  $\gamma_2$  lie between -1 and 1.

Remembering that the stresses are

$$\sigma_x = \frac{\partial^2 \Phi}{\partial y^2}; \quad \sigma_y = \frac{\partial^2 \Phi}{\partial x^2}; \quad \tau_{xy} = -\frac{\partial^2 \Phi}{\partial x \partial y} \quad (64)$$

By substitution of equation (62), we obtain

$$\begin{aligned} \sigma_x &= -(1 - \gamma_1)^2 f''(z + \gamma_1 \bar{z}) - (1 - \gamma_2)^2 g''(z + \gamma_2 \bar{z}) \\ \sigma_y &= (1 + \gamma_1)^2 f''(z + \gamma_1 \bar{z}) + (1 + \gamma_2)^2 g''(z + \gamma_2 \bar{z}) \\ \tau_{xy} &= -i(1 - \gamma_1^2) f''(z + \gamma_1 \bar{z}) - i(1 - \gamma_2^2) g''(z + \gamma_2 \bar{z}) \end{aligned} \quad (65)$$

Introducing the function  $\psi = \psi(x, y)$  by

$$\frac{\partial^2 \psi}{\partial x \partial y} = \nabla_1^2 \Phi, \quad \frac{\partial^2 \psi}{\partial x^2} + \alpha_1 \alpha_2 \frac{\partial^2 \psi}{\partial y^2} = (1 - \alpha_1)(1 - \alpha_2) \frac{\partial^2 \Phi}{\partial x \partial y} \quad (66)$$

the displacements are given by

$$\begin{aligned} u_x &= (b_{12} - b_{22}) \frac{\partial \phi}{\partial x} + b_{11} \frac{\partial \psi}{\partial y} \\ u_y &= (b_{12} - b_{22}) \frac{\partial \phi}{\partial y} + b_{22} \frac{\partial \psi}{\partial x} \end{aligned} \quad (67)$$

From equations (62) and (66), it is possible to obtain

$$\begin{aligned} \frac{\partial \psi}{\partial x} &= -\frac{4i\gamma_1}{1 - \gamma_1} f'(z + \gamma_1 \bar{z}) - \frac{4i\gamma_2}{1 - \gamma_2} g'(z + \gamma_2 \bar{z}) \\ \frac{\partial \psi}{\partial y} &= \frac{4\gamma_1}{1 + \gamma_1} f'(z + \gamma_1 \bar{z}) + \frac{4\gamma_2}{1 + \gamma_2} g'(z + \gamma_2 \bar{z}) \end{aligned} \quad (68)$$

With equations (62) and (68), the displacements given in equation (67) become

$$\begin{aligned} u_x &= (1 + \gamma_1)(b_{12} - \alpha_2 b_{22}) f'(z + \gamma_1 \bar{z}) \\ &\quad + (1 + \gamma_2)(b_{12} - \alpha_1 b_{22}) g'(z + \gamma_2 \bar{z}) \\ u_y &= i(1 - \gamma_1)(b_{12} - \alpha_1 b_{22}) f'(z + \gamma_1 \bar{z}) + \\ &\quad + i(1 - \gamma_2)(b_{12} - \alpha_2 b_{22}) g'(z + \gamma_2 \bar{z}) \end{aligned} \quad (69)$$

Stress function (62) must produce single-valued stresses, which tend to zero at infinity. Also, the displacements must be single-valued. These conditions are satisfied by taking as function  $\phi$  the real part of

$$\begin{aligned} \phi &= f(z + \gamma_1 \bar{z}) + \{A(1 - \gamma_2)(b_{12} - \alpha_2 b_{22}) \\ &\quad + iB(1 + \gamma_2)(b_{12} - \alpha_1 b_{22})\} (z + \gamma_1 \bar{z}) \ln(z + \gamma_1 \bar{z}) \\ &\quad + g(z + \gamma_2 \bar{z}) - \{A(1 - \gamma_1)(b_{12} - \alpha_1 b_{22}) \\ &\quad + iB(1 + \gamma_1)(b_{12} - \alpha_2 b_{22})\} (z + \gamma_2 \bar{z}) \ln(z + \gamma_2 \bar{z}) \end{aligned} \quad (70)$$

where A and B are real constants.



The stresses corresponding to the function (70) are obtained by using equations (64):

$$\begin{aligned}
 \sigma_x &= -(1 - \gamma_1)^2 \left\{ f''(z + \gamma_1 \bar{z}) + [A(1 - \gamma_2)(b_{12} - \alpha_2 b_{22}) \right. \\
 &\quad \left. + iB(1 + \gamma_2)(b_{12} - \alpha_1 b_{22})] \frac{1}{z + \gamma_1 \bar{z}} \right\} \\
 &\quad - (1 - \gamma_2)^2 \left\{ g''(z + \gamma_2 \bar{z}) - [A(1 - \gamma_1)(b_{12} - \alpha_1 b_{22}) \right. \\
 &\quad \left. + iB(1 + \gamma_1)(b_{12} - \alpha_2 b_{22})] \frac{1}{z + \gamma_2 \bar{z}} \right\} \\
 \sigma_y &= (1 + \gamma_1)^2 \left\{ f''(z + \gamma_1 \bar{z}) + [A(1 - \gamma_2)(b_{12} - \alpha_2 b_{22}) \right. \\
 &\quad \left. + iB(1 + \gamma_2)(b_{12} - \alpha_1 b_{22})] \frac{1}{z + \gamma_1 \bar{z}} \right\} \\
 &\quad + (1 + \gamma_2)^2 \left\{ g''(z + \gamma_2 \bar{z}) - [A(1 - \gamma_1)(b_{12} - \alpha_1 b_{22}) \right. \\
 &\quad \left. + iB(1 + \gamma_1)(b_{12} - \alpha_2 b_{22})] \frac{1}{z + \gamma_2 \bar{z}} \right\} \\
 \tau_{xy} &= -i(1 - \gamma_1^2) \left\{ f''(z + \gamma_1 \bar{z}) + [A(1 - \gamma_2)(b_{12} - \alpha_2 b_{22}) \right. \\
 &\quad \left. + iB(1 + \gamma_2)(b_{12} - \alpha_1 b_{22})] \frac{1}{z + \gamma_1 \bar{z}} \right\} \\
 &\quad - (1 - \gamma_2^2) \left\{ g''(z + \gamma_2 \bar{z}) - [A(1 - \gamma_1)(b_{12} - \alpha_1 b_{22}) \right. \\
 &\quad \left. + iB(1 + \gamma_1)(b_{12} - \alpha_2 b_{22})] \frac{1}{z + \gamma_2 \bar{z}} \right\} \quad (71)
 \end{aligned}$$

Now we introduce the two new functions of the complex variable

$$z = re^{i\theta} \quad (72)$$

defined by the integral forms

$$P = \frac{1}{2\pi} \int_0^{2\pi} \frac{ze^{-i\theta} + a}{ze^{-i\theta} - a} (\sigma_r)_{r=a} d\theta$$

$$Q = \frac{i}{2\pi} \int_0^{2\pi} \frac{ze^{-i\theta} + a}{ze^{-i\theta} - a} (\tau_{r\theta})_{r=a} d\theta \quad (73)$$

From these equations, it is possible to see that the functions  $P$  and  $Q$  are related to the tractions on the boundary of the circular hole. In these equations,  $\theta$  is the angular coordinate of the point on the boundary  $r=a$  in which the stresses  $\sigma_r$  and  $\tau_{r\theta}$  are taken. The functions  $P$  and  $Q$  are finite at infinity and

$$\begin{aligned} \{\operatorname{Re}[P]\}_{r=a} &= -(\sigma_r)_{r=a} \\ \{\operatorname{Im}[Q]\}_{r=a} &= -(\tau_{r\theta})_{r=a} \end{aligned} \quad (74)$$

Substituting equations (71) into the transformation formulas

$$\begin{aligned} \sigma_r &= \sigma_x \cos^2 \theta + \sigma_y \sin^2 \theta + 2\tau_{xy} \sin \theta \cos \theta \\ \tau_{r\theta} &= (\sigma_y - \sigma_x) \sin \theta \cos \theta + \tau_{xy} (\cos^2 \theta - \sin^2 \theta) \end{aligned} \quad (75)$$

we obtain the polar components of the stresses:

$$\begin{aligned} \sigma_r &= -\frac{(z - \gamma_1 \bar{z})^2}{z\bar{z}} \left\{ f'(z + \gamma_1 \bar{z}) + [A(1 - \gamma_2)(b_{12} - \alpha_2 b_{22}) \right. \\ &\quad \left. + iB(1 + \gamma_2)(b_{12} - \alpha_1 b_{22})] \frac{1}{z + \gamma_1 \bar{z}} \right\} \\ &\quad - \frac{(z - \gamma_2 \bar{z})^2}{z\bar{z}} \left\{ g''(z + \gamma_2 \bar{z}) - [A(1 - \gamma_1)(b_{12} - \alpha_1 b_{22}) \right. \\ &\quad \left. + iB(1 + \gamma_1)(b_{12} - \alpha_2 b_{22})] \frac{1}{z + \gamma_2 \bar{z}} \right\} \\ \tau_{r\theta} &= -\frac{i(z^2 - \gamma_1^2 \bar{z}^2)}{z\bar{z}} \left\{ f'(z + \gamma_1 \bar{z}) + [A(1 - \gamma_2)(b_{12} - \alpha_2 b_{22}) \right. \\ &\quad \left. + iB(1 + \gamma_2)(b_{12} - \alpha_1 b_{22})] \frac{1}{z + \gamma_2 \bar{z}} \right\} - \end{aligned}$$

(continued)

$$- \frac{i(z - \gamma_2^2 \bar{z}^2)}{z\bar{z}} \left\{ g''(z + \gamma_2 \bar{z}) - [A(1 - \gamma_1)(b_{12} - \alpha_1 b_{22}) + iB(1 + \gamma_1)(b_{12} - \alpha_2 b_{22})] \frac{1}{z + \gamma_2 \bar{z}} \right\} \quad (76)$$

By putting these stresses into equations (74), functions P and Q become

$$\begin{aligned} P = & \frac{1}{a^2} z - \left( \frac{\gamma_1 a^2}{z} \right)^2 f'' \left( z + \frac{\gamma_1 a^2}{z} \right) + \frac{1}{a^2} \left( z - \frac{\gamma_2 a^2}{z} \right) g'' \left( z + \frac{\gamma_2 a^2}{z} \right) \\ & + A(1 - \gamma_2)(b_{12} - \alpha_2 b_{22}) \left( \frac{1 + \gamma_1}{z} - \frac{4\gamma_1}{z + \frac{\gamma_1 a^2}{z}} \right) \\ & - iB(1 + \gamma_2)(b_{12} - \alpha_1 b_{22}) \left( \frac{1 - \gamma_1}{z} + \frac{4\gamma_1}{z + \frac{\gamma_1 a^2}{z}} \right) \\ & - A(1 - \gamma_1)(b_{12} - \alpha_1 b_{22}) \left( \frac{1 + \gamma_2}{z} - \frac{4\gamma_2}{z + \frac{\gamma_2 a^2}{z}} \right) \\ & - iB(1 + \gamma_1)(b_{12} - \alpha_2 b_{22}) \left( \frac{1 - \gamma_2}{z} + \frac{4\gamma_2}{z + \frac{\gamma_2 a^2}{z}} \right) \\ Q = & \frac{1}{a^2} \left( z^2 - \frac{\gamma_1^2 a^4}{z^2} \right) f'' \left( z + \frac{\gamma_1 a^2}{z} \right) - \frac{1}{a^2} \left( z^2 - \frac{\gamma_2^2 a^4}{z^2} \right) g'' \left( z + \frac{\gamma_2 a^2}{z} \right) \\ & + (A + iB)[(1 + \gamma_1)(1 - \gamma_2)(b_{12} - \alpha_2 b_{22}) \\ & - (1 - \gamma_1)(1 + \gamma_2)(b_{12} - \alpha_1 b_{22})] \frac{1}{z} \quad (77) \end{aligned}$$

Now we define the two new variables  $\xi$  and  $\eta$  by the following equations:

$$z = \frac{1}{2} \xi \left[ 1 + \left( 1 - \frac{4\gamma_1 a^2}{\xi^2} \right)^{\frac{1}{2}} \right], \quad z = \frac{1}{2} \eta \left[ 1 + \left( 1 - \frac{4\gamma_2 a^2}{\eta^2} \right)^{\frac{1}{2}} \right] \quad (78)$$

The variable  $z$  must be single-valued. Therefore, the arguments of the complex expressions  $1 - 4\gamma_1 a^2/\xi^2$  and  $1 - 4\gamma_1 a^2/\eta^2$  must be between  $-\pi$  and  $\pi$ .

Combining equations (77) and (78), we get

$$\begin{aligned}
 & (\gamma_1 - \gamma_2) \left( 1 - \frac{4\gamma_1 a^2}{\xi^2} \right)^{\frac{1}{2}} [\xi f'(\xi) + A(1 - \gamma_2)(b_{12} - \alpha_2 b_{22}) \\
 & + iB(1 + \gamma_2)(b_{12} - \alpha_1 b_{22})] = A(1 - \gamma_2)^2 (b_{12} - \alpha_2 b_{22}) \\
 & - A(1 - \gamma_1)(1 - \gamma_2)(b_{12} - \alpha_1 b_{22}) - iB(1 + \gamma_2)^2 (b_{12} - \alpha_1 b_{22}) \\
 & + iB(1 + \gamma_1)(1 + \gamma_2)(b_{12} - \alpha_2 b_{22}) \\
 & - \frac{\xi}{4} [P(\xi) + Q(\xi)] \left[ 1 + \left( 1 - \frac{4\gamma_1 a^2}{\xi^2} \right)^{\frac{1}{2}} \right] \\
 & - \frac{\gamma_2 \xi}{4\gamma_1} [P(\xi) - Q(\xi)] \left[ 1 - \left( 1 - \frac{4\gamma_1 a^2}{\xi^2} \right)^{\frac{1}{2}} \right] \quad (79)
 \end{aligned}$$

$$\begin{aligned}
 & (\gamma_1 - \gamma_2) \left( 1 - \frac{4\gamma_2 a^2}{\eta^2} \right)^{\frac{1}{2}} [\eta g'(\eta) - A(1 - \gamma_1)(b_{12} - \alpha_1 b_{22}) \\
 & - iB(1 + \gamma_1)(b_{12} - \alpha_2 b_{22})] = A(1 - \gamma_1)^2 (b_{12} - \alpha_1 b_{22}) \\
 & - A(1 - \gamma_1)(1 - \gamma_2)(b_{12} - \alpha_2 b_{22}) - iB(1 + \gamma_1)^2 (b_{12} - \alpha_2 b_{22}) \\
 & + iB(1 + \gamma_1)(1 + \gamma_2)(b_{12} - \alpha_1 b_{22}) \\
 & + \frac{\eta}{4} [P(\eta) + Q(\eta)] \left[ 1 + \left( 1 - \frac{4\gamma_2 a^2}{\eta^2} \right)^{\frac{1}{2}} \right] \\
 & + \frac{\gamma_1 \eta}{4\gamma_2} [P(\eta) - Q(\eta)] \left[ 1 - \left( 1 - \frac{4\gamma_2 a^2}{\eta^2} \right)^{\frac{1}{2}} \right] \quad (80)
 \end{aligned}$$

For a particular problem, functions  $P$  and  $Q$  can be computed by using equations (74), because  $(\sigma_r)_{r=a}$  and  $(\tau_{r\theta})_{r=a}$  are known from the boundary conditions. By integration of the stresses on the boundary of the hole, we find the resultant forces

$$\begin{aligned} X &= 2\pi A(1 - \gamma_1)(1 - \gamma_2)(\alpha_1 - \alpha_2)b_{22} \\ Y &= 2\pi B(1 + \gamma_1)(1 + \gamma_2)(\alpha_1 - \alpha_2)b_{22} \end{aligned} \quad (81)$$

Equations (73) can be used to find  $V(z)$  and  $W(z)$ , and by solving equations (81), constants  $A$  and  $B$  can be established. Then, from equations (79) and (80), we obtain  $f'(\xi)$  and  $g'(\eta)$ . The stresses are given by equations (71), where we now have

$$\xi = z + \gamma_1 \bar{z}, \quad \eta = z + \gamma_2 \bar{z} \quad (82)$$

#### Stress Concentration for Pure Tension

The tension load  $T_x$  is applied at the  $x$  direction, as shown in Figure 43.

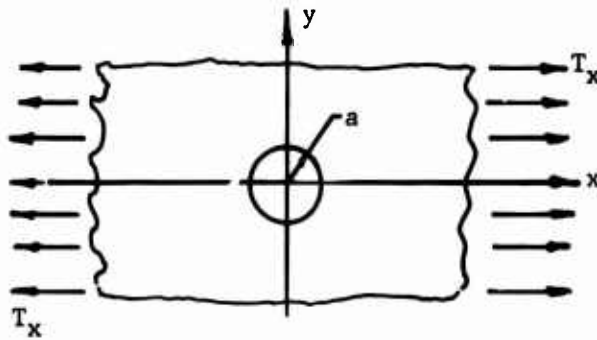


Figure 43. Tension Load Applied at the  $x$  Direction.

If there is no hole in the plate, the stresses are

$$\sigma_x = T_x, \quad \sigma_y = 0, \quad \tau_{xy} = 0 \quad (83)$$

or, in polar coordinates, according to the formulas (75),

$$\sigma_r = \frac{T_x}{2} (1 + \cos 2\theta)$$

$$\begin{aligned}
\sigma_{\theta} &= \frac{T_x}{2} (1 - \cos 2\theta) \\
\tau_{r\theta} &= -\frac{T_x}{2} \sin 2\theta
\end{aligned} \tag{84}$$

It is now necessary to find a stress system which tends to zero at the infinite and which takes the values

$$\begin{aligned}
(\sigma_r)_s &= -\frac{T_x}{2} (1 + \cos 2\theta) \\
(\sigma_{\theta})_s &= -\frac{T_x}{2} (1 - \cos 2\theta) \\
(\tau_{r\theta})_s &= \frac{T_x}{2} \sin 2\theta
\end{aligned} \tag{85}$$

when  $r = a$

Thus, from equations (73), we get

$$P = \frac{T_x}{2} \left(1 + \frac{a^2}{z^2}\right), \quad Q = -\frac{T_x}{2} \left(1 - \frac{a^2}{z^2}\right) \tag{86}$$

With the variables  $\xi$  and  $\eta$  introduced by equations (78), we obtain the following from equations (84):

$$\begin{aligned}
P(\xi) - Q(\xi) &= P(\eta) - Q(\eta) = T \\
P(\xi) + Q(\xi) &= \frac{T_x \left\{ 1 - \left( 1 - \frac{4\gamma_1 a^2}{\xi^2} \right)^{\frac{1}{2}} \right\}}{\gamma_1 \left\{ 1 + \left( 1 - \frac{4\gamma_1 a^2}{\xi^2} \right)^{\frac{1}{2}} \right\}} \\
P(\eta) + Q(\eta) &= \frac{T_x \left\{ 1 - \left( 1 - \frac{4\gamma_2 a^2}{\eta^2} \right)^{\frac{1}{2}} \right\}}{\gamma_2 \left\{ 1 + \left( 1 - \frac{4\gamma_2 a^2}{\eta^2} \right)^{\frac{1}{2}} \right\}}
\end{aligned} \tag{87}$$

Because the stresses in equation (85) do not produce a resultant force on the hole, constants A and B are zero, as equations (81) demonstrate. Thus, from equations (79) and (80), and by taking into account equations (77) and (87), we get

$$\begin{aligned} f''(\xi) &= \frac{T_X(1 + \gamma_2)}{4\gamma_1(\gamma_1 - \gamma_2)} \left\{ 1 - \left( 1 - \frac{4\gamma_1 a^2}{\xi^2} \right)^{-\frac{1}{2}} \right\} \\ g''(\eta) &= - \frac{T_X(1 + \gamma_1)}{4\gamma_2(\gamma_1 - \gamma_2)} \left\{ 1 - \left( 1 - \frac{4\gamma_2 a^2}{\eta^2} \right)^{-\frac{1}{2}} \right\} \end{aligned} \quad (88)$$

Using these equations and remembering equations (82), then the following stresses result from equations (71):

$$\begin{aligned} \sigma_x &= \frac{(1 + \gamma_1)(1 + \gamma_2)T_X}{4\gamma_1\gamma_2} + \frac{T_X}{4(\gamma_1 - \gamma_2)} \left\{ \frac{(1 + \gamma_2)(1 - \gamma_1)^2}{\gamma_1 X_1} - \frac{(1 + \gamma_1)(1 - \gamma_2)^2}{\gamma_2 X_2} \right\} \\ \sigma_y &= - \frac{(1 + \gamma_1)(1 + \gamma_2)T_X}{4\gamma_1\gamma_2} - \frac{T_X}{4(\gamma_1 - \gamma_2)} \left\{ \frac{(1 + \gamma_2)(1 + \gamma_1)^2}{\gamma_1 X_1} - \frac{(1 + \gamma_1)(1 + \gamma_2)^2}{\gamma_2 X_2} \right\} \\ \tau_{xy} &= \frac{iT_X}{4(\gamma_1 - \gamma_2)} \left\{ \frac{(1 + \gamma_2)(1 - \gamma_1^2)}{\gamma_1 X_1} - \frac{(1 + \gamma_1)(1 - \gamma_2^2)}{\gamma_2 X_2} \right\} \end{aligned} \quad (89)$$

where the following notation was used:

$$X_1 = R_1^{\frac{1}{2}} e^{\frac{i\phi_1}{2}}, \quad X_2 = R_2^{\frac{1}{2}} e^{\frac{i\phi_2}{2}} \quad (90)$$

with

$$R_1 e^{i\phi_1} = 1 - \frac{4a^2\gamma_1}{(z + \gamma_1\bar{z})^2}, \quad R_2 e^{i\phi_2} = 1 - \frac{4a^2\gamma_2}{(z + \gamma_2\bar{z})^2} \quad (91)$$

The quantities  $R_1$  and  $R_2$  are chosen as positive, and  $\phi_1$  and  $\phi_2$  are to lie between  $-\pi$  and  $\pi$ . Also, it must be emphasized that the actual stresses are the real part of complex expressions (89).

#### Stress Concentration for Pure Shear

A shear load  $T_{xy}$  is applied on the plate with the circular hole, as shown in Figure 44. If the hole is absent, the stresses are

$$\sigma_x = 0, \quad \sigma_y = 0, \quad \tau_{xy} = T_{xy} \quad (92)$$

or, in polar coordinates,

$$\begin{aligned} \sigma_r &= T_{xy} \sin 2\theta \\ \sigma_\theta &= -T_{xy} \sin 2\theta \\ \tau_{r\theta} &= T_{xy} \cos 2\theta \end{aligned} \quad (93)$$

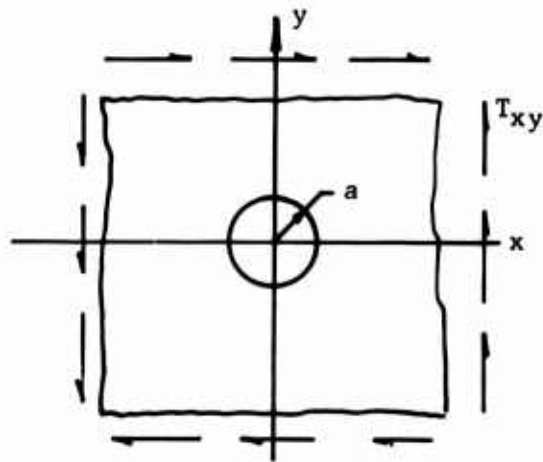


Figure 44. Shear Load Applied on Plate With Circular Hole.

Thus, to keep the hole boundary free of stresses, it is necessary to sum to the stresses (93), the stresses which take the values

$$(\sigma_r)_s = -T_{xy} \sin 2\theta, \quad (\tau_{r\theta})_s = -T_{xy} \cos 2\theta \quad (94)$$

on the hole and zero at the infinite.



By using equations (73), we obtain

$$P = Q = T_{xy} \frac{ia^2}{z^2} \quad (95)$$

and, from equations (84),

$$P(\xi) - Q(\xi) = P(\eta) - Q(\eta) = 0 \quad (96)$$

$$\begin{aligned} P(\xi) + Q(\xi) &= \frac{2i T_{xy} \left\{ 1 - \left( 1 - \frac{4\gamma_1 a^2}{\xi^2} \right)^{\frac{1}{2}} \right\}}{\gamma_1 \left\{ 1 + \left( 1 - \frac{4\gamma_1 a^2}{\xi^2} \right)^{\frac{1}{2}} \right\}} \\ P(\eta) + Q(\eta) &= \frac{2i T_{xy} \left\{ 1 - \left( 1 - \frac{4\gamma_2 a^2}{\eta^2} \right)^{\frac{1}{2}} \right\}}{\gamma_2 \left\{ 1 + \left( 1 - \frac{4\gamma_2 a^2}{\eta^2} \right)^{\frac{1}{2}} \right\}} \end{aligned} \quad (97)$$

The constants A and B of equations (81) are zero because the resultant force (X, Y) on the hole is zero. With equations (96) and (97), from equations (79) and (80), we have

$$\begin{aligned} f''(\xi) &= \frac{i T_{xy}}{2\gamma_1 (\gamma_1 - \gamma_2)} \left\{ 1 - \left( 1 - \frac{4\gamma_1 a^2}{\xi^2} \right)^{-\frac{1}{2}} \right\} \\ g''(\eta) &= \frac{i T_{xy}}{2\gamma_2 (\gamma_1 - \gamma_2)} \left\{ 1 - \left( 1 - \frac{4\gamma_2 a^2}{\eta^2} \right)^{\frac{1}{2}} \right\} \end{aligned} \quad (98)$$

With these results and with equations (82), from equations (71) the stresses are the real part of the expressions

$$\begin{aligned} \sigma_x &= \frac{i T_{xy}}{2(\gamma_1 - \gamma_2)} \left\{ \frac{(1 - \gamma_1)^2}{\gamma_1 X_1} - \frac{(1 - \gamma_2)^2}{\gamma_2 X_2} \right\} \\ \sigma_y &= - \frac{i T_{xy}}{2(\gamma_1 - \gamma_2)} \left\{ \frac{(1 + \gamma_1)^2}{\gamma_1 X_1} - \frac{(1 + \gamma_2)^2}{\gamma_2 X_2} \right\} \end{aligned} \quad (99)$$

$$\tau_{xy} = \frac{\gamma_1 \gamma_2 - 1}{2\gamma_1 \gamma_2} T_{xy} - \frac{T_{xy}}{2(\gamma_1 - \gamma_2)} \left\{ \frac{1 - \gamma_1^2}{\gamma_1 X_1} - \frac{1 - \gamma_2^2}{\gamma_2 X_2} \right\} \quad (100)$$

The meaning of  $X_1$  and  $X_2$  is given in equations (90) and (91).

#### Numerical Results

By using equations (89) and (99), it is possible to study the stress concentration around a circular hole on an orthotropic plate under axial and shear loads. A computer program based on these complex expressions was developed.

A representative value of the stress concentration originated by a circular hole is the stress  $(\sigma_x)_m$  at the point A when the applied load produces unit stress in the plate without a hole. By using the computer program, we obtained the results plotted in Figure 45. It can be observed that the maximum stress concentration factor is obtained when the axial load is applied in the direction of the fibers (x, in this case).

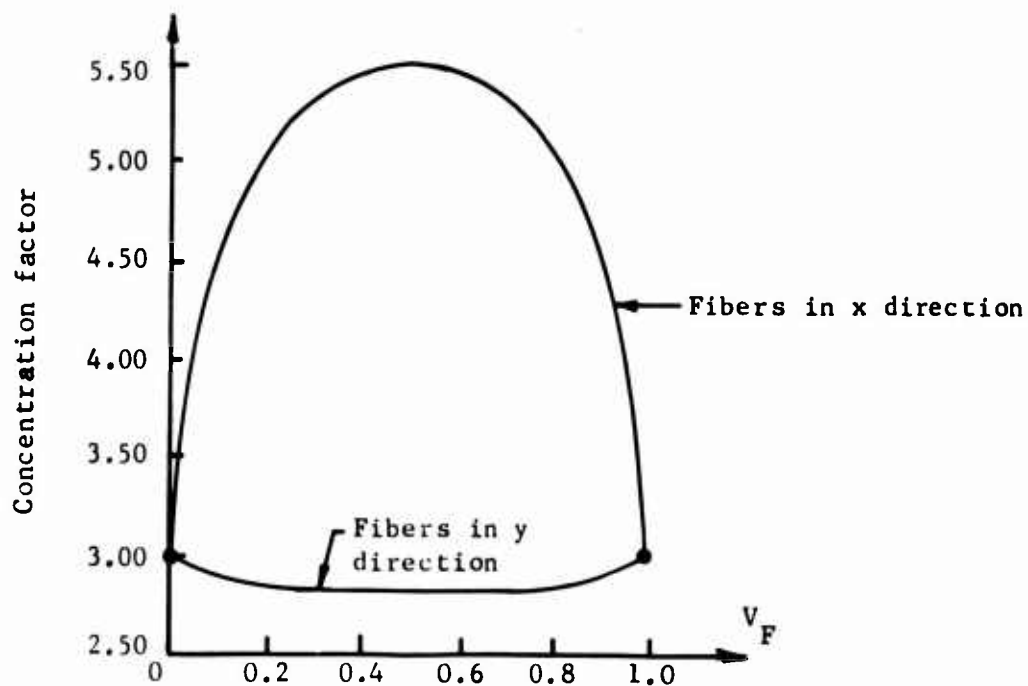


Figure 45. Stress Concentrations (maximum stresses  $\sigma_{xm}$ ) in a Unidirectional Composite With a Circular Hole.

The magnitude of the concentration factor indicates that this phenomenon is more critical than in the isotropic materials, where the factor is only 3.

It must be noted that this analysis is based on the hypothesis that the material constants do not depend on  $x$  and  $y$ . To satisfy this condition, the hole must be made by cutting the fibers as indicated in Figure 46.

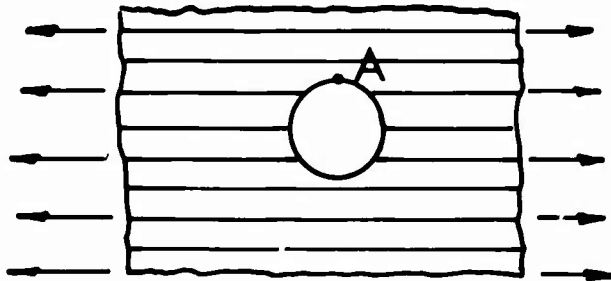


Figure 46. Orthotropic Sheet.

A more rational form for the fibers is shown in Figure 47, where fibers are not cut. The corresponding stress analysis cannot be performed with expressions (89) and (99) because now the elastic constants change from point to point. In other words, the material is not quasi-homogeneous. However, the stress analysis of this case can be made by using the method developed on page 46.

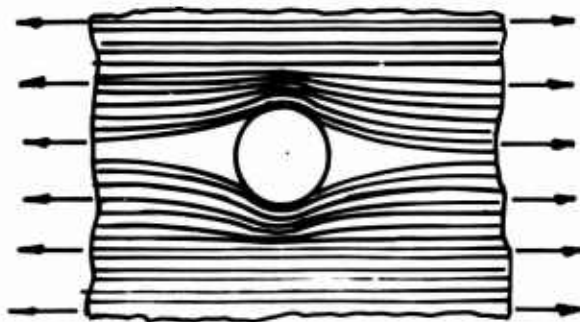


Figure 47. Nonorthotropic Sheet.

The numerical study was performed for unidirectional composites, in which the importance of the stress concentration factor is more evident. However, formulas (89) and (99) can also be applied for multilayered composite materials by considering the adequate constants  $b_{ij}$ .

## CONCLUSIONS AND RECOMMENDATIONS

The introduction of localized high loads into a thin sheet can be accomplished effectively by the use of bolt joints; contrary to the metallic joints, there are no discontinuities present when joints made of filamentary composites are used. A further advantage of filamentary joints is their behavior when loaded to failure. For example, after the first break occurs, the joint can still be loaded to almost twice the failure load. Since the first break is very easily detectable, it is possible to avoid continuation of a catastrophic failure and still maintain the load-carrying capability of a structure.

The stress analysis of composite joints, as presented herein, shows that an effective safety factor can be obtained that is within the accuracy demanded by technical requirements.

In a circular form cutout in a unidirectional composite, the stresses can reach values twice as high as those attained in an isotropic material if the fibers terminate in the rim of the cutout. For this reason, it is recommended that the fibers be led in a continuous manner around the cutout as far as fabrication requirements permit. Impelled by the great potentials of composite joints, further studies of other shapes of joints and types of loadings are recommended.

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13. ABSTRACT <p>In this report the stress distribution in loop and bolt joints made of composite materials is studied using analytical and experimental methods. Also, the stress concentration in orthotropic composite sheets with a circular hole is studied.</p> <p>The numerical technique to evaluate the stresses and displacements is described. Simple design formulas to compute the stresses at critical regions are developed. A procedure of structural design is delineated and applied to specific cases.</p> <p>Experimental results obtained by testing composite joints are reported. Stress distribution tests were performed using the photoelastic technique. Failure location and ultimate loads for several composite joints are shown.</p> <p>This report contains the analysis of the stress concentration that appears around a circular hole in an orthotropic composite sheet under simple tension and simple shear loadings.</p>		

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